



Wash Creek Watershed

STORMWATER MASTER PLAN

May 2019

WR Job Number: 2180508

Jennifer Diaz, PE

INTRODUCTION

The City of Hendersonville has engaged WithersRavenel for the development of a stormwater master plan for the Wash Creek watershed. The Wash Creek Watershed is approximately 1378 acres. The goal of the stormwater master plan is to identify and prioritize capital improvement projects that will allow the City to plan and budget for improvements to its stormwater infrastructure.

FIELD RECONNAISSANCE SUMMARY

The preliminary data analysis utilized existing GIS data provided by the City to determine the structures within the Wash Creek Watershed that would require additional inspection. Field data collection was done in the Fall of 2018. The field crews collected information on the stormwater structures by pulling lids and making visual observations from ground level. A total of 641 structures were surveyed for additional information (a total of 778 structures were identified but 137 of them were inaccessible). The team was able to obtain measurements and note the general condition, all of which is documented in the ArcGIS database. The condition of each structure was categorized as either Excellent, Good, Fair, Poor, or Needs Repair.

The observed conditions are defined as follows:

- Excellent** New infrastructure in perfect condition;
- Good** Infrastructure with no discernable flaws, in optimal working condition without much sediment or debris;
- Fair** Infrastructure is in working order but shows signs of age. There is a significant amount of wear and tear and/or debris and sediment is substantial;
- Poor** Infrastructure needs maintenance (signs of an active problem such as cracking, sedimentation, etc). These structures are visibly broken or in imminent danger of failure;
- Needs Repair** The infrastructure has failed and requires immediate maintenance action.

The table below tabulates the condition assessments of the structures:

Table 1. Field results of condition assessment

Condition	Number of Structures
Excellent	1
Good	281
Fair	241
Poor	103
Needs Repair	15

Additional comments are included in the ArcGIS data when the condition is considered to be Poor or Needs Repair.

Inventory Analysis

The GIS inventory located inlets, manholes, and culvert endwalls/headwalls. The connectivity of these elements in the GIS interface determined pipe length and slope. The Wash Creek Watershed was determined to have 49,405 linear feet of storm drainage pipe; 1,671 linear feet of that is associated with culverts. Since the pipes themselves were not inspected, a condition is not directly attributed to them in the GIS database. For the purposes of this analysis, a condition was attributed to the pipe based on the worse condition of the connected structures. For instance, if the upstream inlet of the pipe was considered to be in “fair” condition and the downstream inlet was considered to be in “good” condition, the pipe was assigned a “fair” condition.

Culvert Diameter

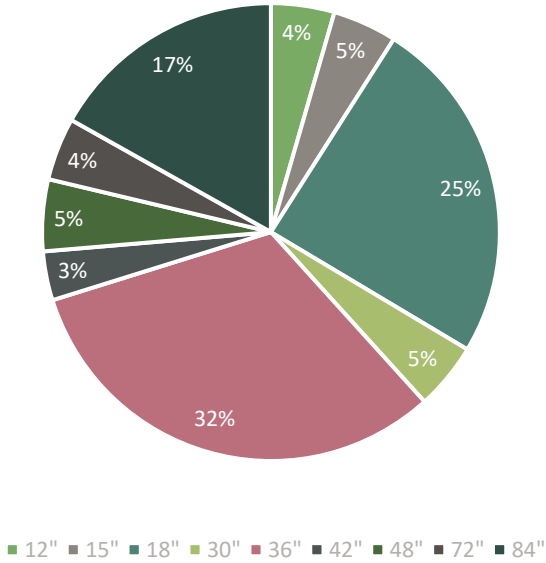


Figure 1. Diameter of culverts in inches by percentage of linear foot

Pipe Diameter

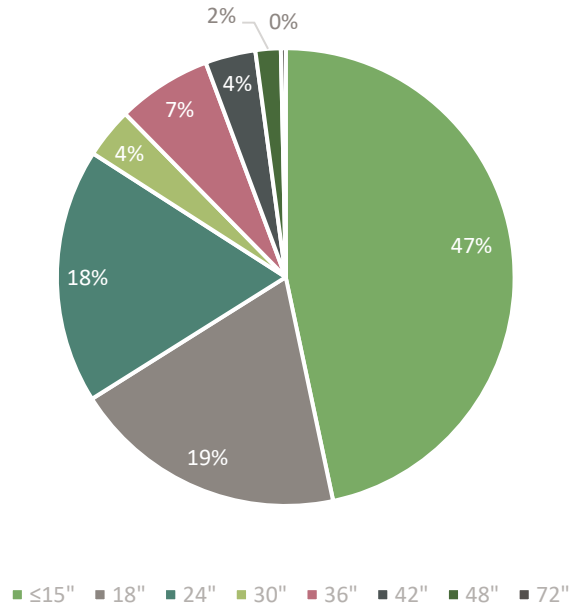


Figure 2. Diameter of pipes in inches by percentage of linear foot

Installation information of the infrastructure was not available. Therefore, age was estimated based on the material and likely timeframe that material was installed in this region. For instance, corrugated metal pipes were quite popular in the 1970s therefore metal pipes were assumed to be between 40-49 years old.

Culvert Age

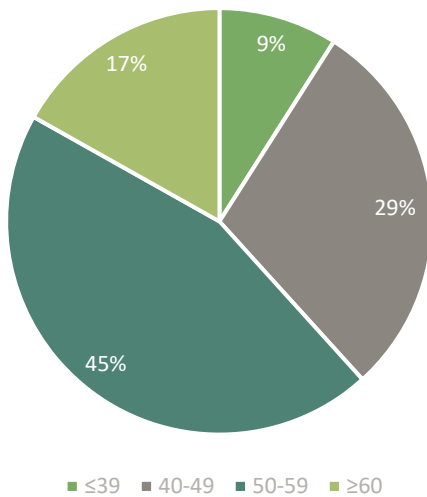


Figure 3. Estimated age of culverts in years by percentage of linear foot

Pipe Age

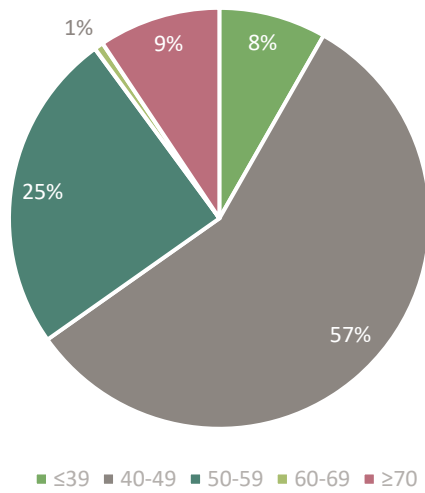


Figure 4. Estimated age of pipes in years by percentage of linear foot

STORMWATER INFRASTRUCTURE ASSESSMENT

Each of the elements of the surveyed stormwater network were evaluated in order to prioritize repair or replacement. Based on the stormwater inventory, the attributes stored in the GIS database were used to establish a “score”. The score was based on the “Likelihood of Failure” (LoF) and “Consequence of Failure” (CoF).

Likelihood of Failure or LoF was based on condition, age, and material. Condition was determined from the field assessment. Age was estimated based on the material as described above. Material was determined from the field observations.

The CoF attempts to quantify the criticality of the infrastructure. The CoF was based on diameter and general location. Larger diameter pipes were assigned a higher score. Infrastructure located within a road was assigned a higher score than those located outside the roadway.

Lower scores for LoF and CoF indicate less risk, while higher scores indicate greater risk. Together, combining the condition and the consequence of failure of the structures allowed for a general prioritization score to be determined for each structure. The overall rating indicates the relative priority for repair/replacement of the stormwater infrastructure element.

The tables below illustrate the numeric score that was assigned based on the associated criteria:

Table 2. Likelihood of Failure (LoF) score based on estimated age of pipe.

Pipe Age (years)	LoF Score
60+	7
50+	6
40+	5
30+	4

Table 3. Likelihood of Failure (LoF) score based on material of pipe.

Material	LoF Score
PVC	0.25
CMP	0.5
CPP	0.25
Concrete	0.5
Clay	1
Brick	1
DIP	0.5

Table 4. Likelihood of Failure (LoF) score based on condition of pipe.

Condition	LoF Score
Good	0.5
Fair	1
Poor	2

Table 5. Consequence of Failure (CoF) score based on diameter of pipe.

Diameter (inches)	CoF Score
50+	8
40 - 50	7
30 - 40	6
20 - 30	5
15 - 18	4
6 - 12	3
Unknown	2

Table 6. Consequence of Failure (CoF) score based on location of pipe.

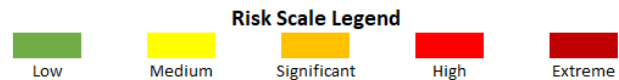
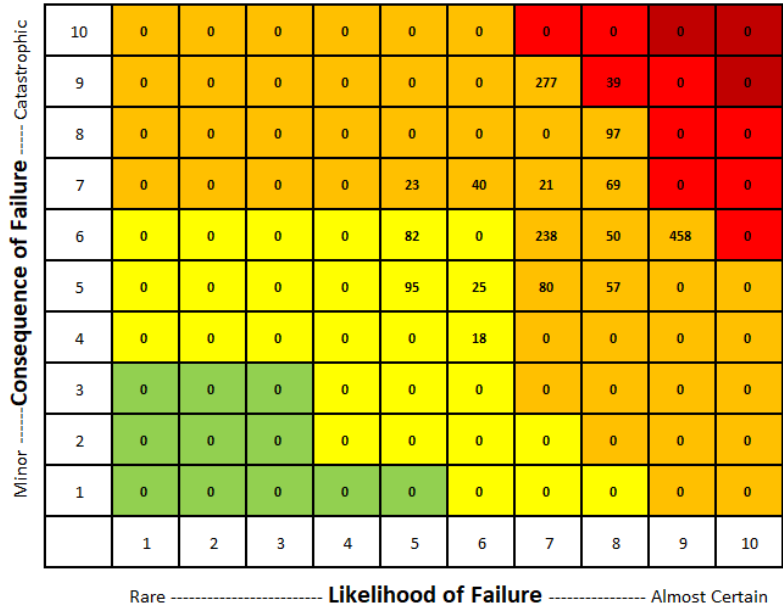
Within a Roadway	CoF Score
Yes	2
No	1

Structure Prioritization

The LoF and CoF scores were tabulated for each individual structure. Each structure has been assigned this priority score in the GIS database. The scores range from 1-100, from least priority to most priority. The scores were gradated from low priority to extreme priority. The following charts summarize the results of the prioritization data analysis. Maps depicting the location of structures and identifying them by priority have been included in the Maps section of this report.

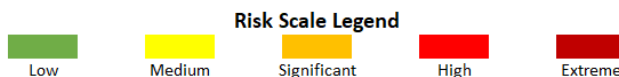
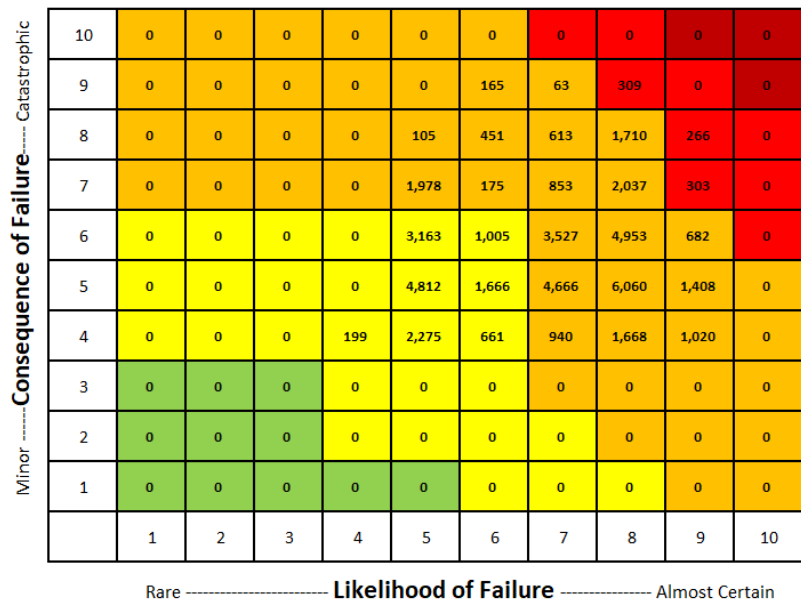
Culverts

- Medium Priority – 221 linear feet
- Significant Priority – 1,411 Linear feet
- High Priority – 39 Linear feet



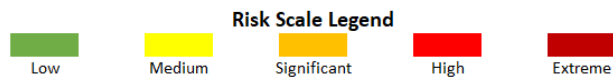
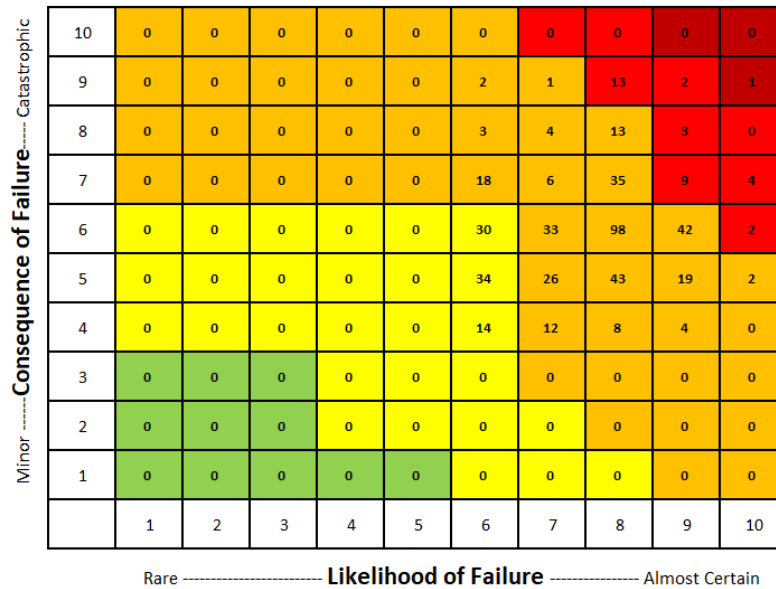
Pipes

- Medium Priority – 13,782 linear feet
- Significant Priority – 33,075 Linear feet
- High Priority – 877 Linear feet



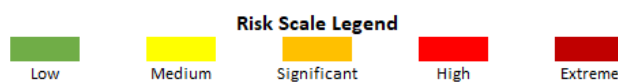
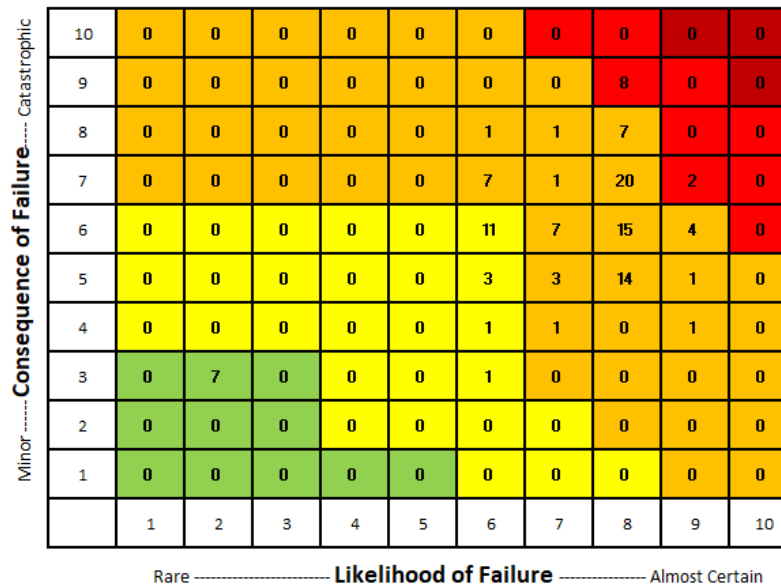
Inlets

- Medium Priority – 78
- Significant Priority – 369
- High Priority – 33
- Extreme Priority – 1



Manholes

- Low Priority - 7
- Medium Priority – 16
- Significant Priority – 98
- High Priority – 10



POTENTIAL PROJECT IDENTIFICATION

Areas of Concern

From the field reconnaissance, with input from the City, five (5) areas of concern were identified. They are listed below. Please refer to the Area of Concern identification map for further information. From these areas of concern, with consideration of the prioritization analysis, five (5) project areas were identified for further study.

Area of Concern #1 *Along 5th Ave W from Blythe Street and Ehringhaus St / Along Valley St from Holmes Street to 3rd Ave W*

This area is approximately 26.25 acres of the watershed. The following infrastructure was located within this area:

Structure	Quantity (pipes in lf)
15" RCP	1215
18" RCP	1029
24" RCP	1041
30" RCP	450
36" RCP	38
42" RCP	24
48" RCP	63
Small Box Culvert	1058
Manholes	7
Inlets	51
Headwalls	6

Of the infrastructure listed above 26 inlets were found to be in fair condition, 8 in poor condition, and 2 were in need of repair. The remaining inlets were considered to be in good condition. One of the inlets identified as in need of repair, INLEI 301, was noted as a potentially hazardous condition in its current state. The other in need of repair inlet, INLE302 was noted to have the grate cemented on.

There are 6 discharge points in this area. One was considered fair condition and 2 to be in poor condition. The remaining discharge points were found to be in good condition.

There are 7 manholes in this area. One was considered to be in fair condition and 3 were unknown. The remaining 3 were in good condition.

The prioritization analysis identified the infrastructure within this area of concern to be high priority. Three (3) areas were identified for further study; Project Areas #1A, #1B, and #1C were analyzed as potential improvement projects.

Area of Concern #2 *Around Buncombe Street between 5th Ave W and Lily Pond Dr*

This area is approximately 29.28 acres of the watershed. The following infrastructure was located within this area:

Structure	Quantity (pipes in lf)
15" RCP	2233
18" RCP	704
24" RCP	577
30" RCP	890
36" RCP	675
48" RCP	214
Small Box Culvert	596
Large Box Culvert	68
Manholes	13
Inlets	45
Headwalls	7

Of the infrastructure listed above, 17 inlets were found to be in fair condition, 10 in poor condition, and 8 were inaccessible and/or unknown. The remaining inlets were considered to be in good condition.

There are 7 discharge points in this area. Three were considered to be in fair condition and 3 were found to be in poor condition. One discharge point was considered to be in good condition. Of note, 2 of the discharge points identified as in fair condition, STD73 and STDP 74, appear to be conveying stormwater underneath houses.

There are 13 manholes in this area. One was considered to be in fair condition and 7 were unknown. The remaining 5 were in good or excellent condition.

The prioritization analysis identified the infrastructure within this area of concern to be high priority. Two (2) areas were identified for further study; Project Areas #2A and #2B.

Area of Concern #3 *Near 5th and 3rd Ave West between Jordan St and Blythe St*

This area is approximately 10.97 acres of the watershed. The following infrastructure was located within this area:

Structure	Quantity (pipes in lf)
15" RCP	1137
18" RCP	311
24" RCP	215
30" RCP	209
36" RCP	64
Large Box Culvert	30
Manholes	6
Inlets	19
Headwalls	7

Of the infrastructure listed above, 5 inlets were found to be in fair condition, 6 in poor condition, and 3 were in need of repair. The remaining inlets were considered to be in good condition. The inlets in need of repair were identified as INLE426, INLE 439, and INLE 442.

There are 7 discharge points in this area. One was considered to be in fair condition and 3 were found to be in poor condition. The remaining three discharge points were considered to be in good condition.

There are 6 manholes in this area. Two were considered to be in fair condition, 2 in poor condition, and 2 were unknown.

The prioritization analysis identified the structures in this area to be medium priority in general. The prioritization analysis did identify one portion of the system (GRAV 429) as being high/significant priority in this area.

Area of Concern #4 (PW #1) Near Blythe Street between Pinecrest Drive and Glenbrook Drive

This area is approximately 5.45 acres of the watershed. The following infrastructure was located within this area:

Structure	Quantity (pipes in lf)
15" RCP	117
18" RCP	336
Inlets	5
Headwalls	1

Of the infrastructure listed above, one inlet was found to be in fair condition and one was in need of repair. The remaining inlets were considered to be in good condition. The inlet in need of repair was identified as INLE445. It was noted that the lid was stuck and possibly abandoned.

There was 1 discharge points in this area. It was found to be in good condition.

Evaluation of this area identified an open conveyance feature at the rear of residential lots and a pond. The prioritization analysis identified the structures in this area to be low priority.

Area of Concern #5 (PW#2) Between N Oak Street and N Justice Street from Florida Ave and 4th Ave West

This area is approximately 2.86 acres of the watershed. The following infrastructure was located within this area:

Structure	Quantity (pipes in lf)
24" RCP	149
Small Box Culvert	40
Manholes	1
Inlets	4
Headwalls	3

Of the infrastructure listed above, 2 inlets were found to be in fair condition and 1 was unknown. The remaining inlets were considered to be in good condition.

There are 3 discharge points in this area. One was considered to be in fair condition and 2 were found to be in good condition.

The condition of the manhole in this area could not be verified.

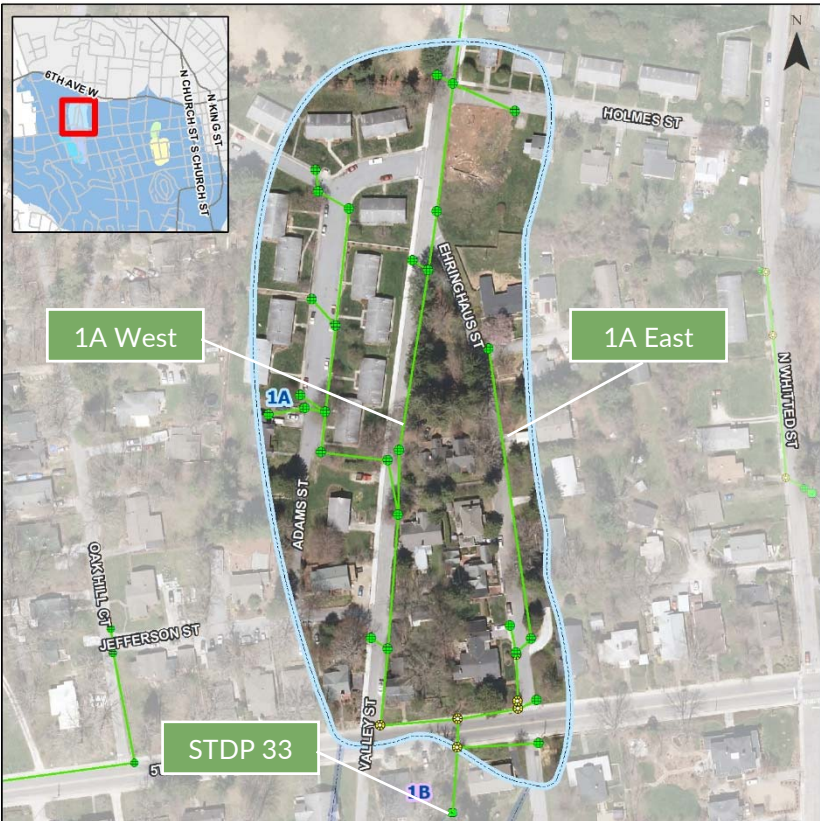
The prioritization analysis identified the structures in this area to be low priority.

Project Area Analysis

Five (5) project areas were identified for further study. Simplified hydrologic and hydraulic models were created for each project area based on the collected survey information, estimated drainage area, and land use assumptions. Assumptions and adjustments were made for the purposes of this study for inverts where survey data was unknown or incomplete. PCSWMM was used to evaluate the project area drainage systems using modified rational for flows simulating the 10-year storm event. The purpose of this exercise was to understand the potential conveyance capacity of the existing system. This is for informational purposes only. Profile results are included in the Project Area Analysis section of the report.

Project Area 1A

This system outlets to STDP 33. There are two main lines of storm drainage that contribute to this outfall:



1A West and 1A East. The drainage area to 1A West was determined to be 14 acres. The drainage area to 1A East was determined to be 7 acres. Both systems are piped systems without open channels. Each of these systems is located within the right-of-way.

The conveyance analysis results show that the storm drainage outfall where these two systems meet is adequately sized for the modeled flows however the condition may require replacement. The upstream systems are not sized to convey the design storm and should be upsized.

Recommendation: Both pipe systems 1A East and 1A West should be upgraded in order to convey the design storm. Site evaluation of the culvert at STDP 33 is recommended to determine if the pipe should be replaced.

**Stormwater Master Plan
Wash Creek Watershed
Hendersonville, North Carolina**

Project Area 1B

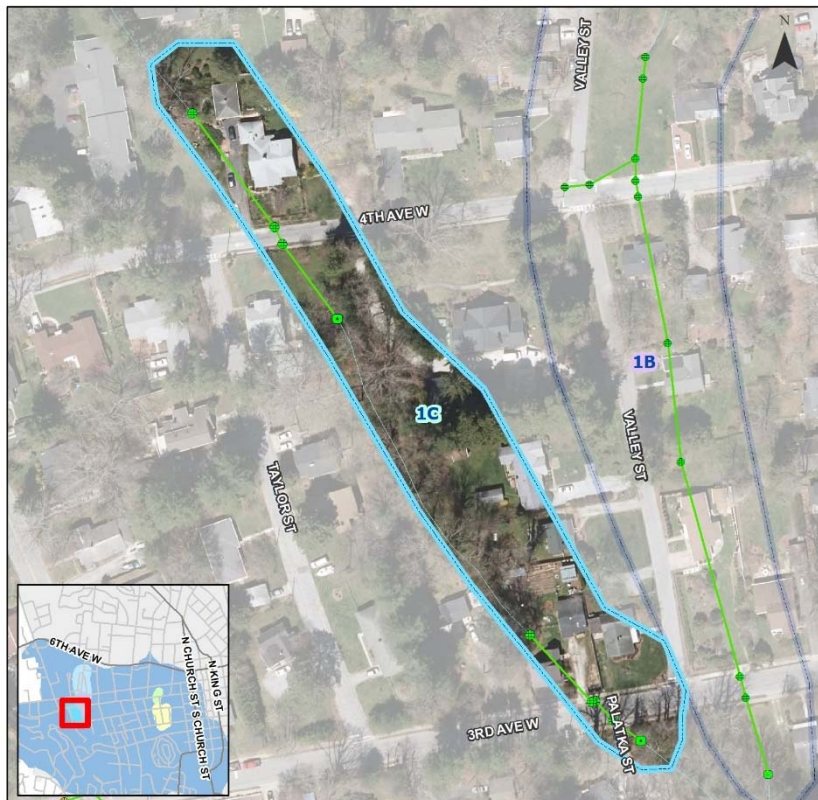
This system is just downstream of Project Area 1A. The drainage area totals 29 acres. The system is comprised of an open channel from discharge point at STDP 33 which then transitions to a subsurface network of 24" VCP and concrete box culverts. The system traverses through private property.

The system is not adequately sized to handle the design storm as modeled.

Recommendation:

It is recommended that the materials and pipe size be upgraded as well as rerouting the system to stay within the right-of-way.

This recommendation requires the addition of a minimum of 2 junction boxes as well as an



additional 100 linear feet of storm drainage.

Project Area 1C

This system receives a drainage area of 70 acres. The system is comprised of channel flow as well as a piped network. It is located primarily on private property.

The system is not adequately sized to handle the design storm as modeled.

Recommendation:

It is recommended that the conveyance system be rerouted to stay within the right-of-way. This recommendation requires the addition of a minimum of 2 junction boxes as well as an additional 600 linear feet of storm drainage.

Alternatively, channel improvements could be made if approved by environmental permitting agencies and the channel system could be placed in an easement to be operated and maintained by the City. Improvements to the storm drain network associated with this project site would still be warranted if this alternative is explored.

Project Area 2A

Similar to Project 1A, Project 2A is comprised of two main storm drain networks that converge at one outfall point. Each system was analyzed as 2 distinct systems; Project 2A East and Project 2A West. The drainage area to Project 2A East is 32 acres while the drainage area to Project 2A West is 38 acres. Project 2A East is a piped system where as the Project 2A West has portions of open channel. Both systems are located on private property with portions crossing the right-of-way.



Project 2A East is undersized for the majority of network. The pipe size is reduced midway through which restricts flow.

Project 2A West is undersized up until the stream. Then the stream and downstream infrastructure adequately handle the flow in the design storm. The undersized portion has been identified separately as Project 2B below.

Recommendation: Improvements to the 2A should include upgrading the pipe that is currently restricting flow. This 15" pipe goes through an open space area. It is recommended that the pipe size be upgraded throughout this network and that the pipe through the open area be realigned through the nearby parking lot that then ties into the right-of-way. Easements may be required.

It is also recommended that the drainage systems be located into the right-of-way. The west alignment is proposed to move into Buncombe Street. The East alignment is proposed to go into Rhodes Street. This adds approximately 195 LF and 4 structures for the realignment along Rhodes Street and 270 LF and 1 additional structure for the realignment along Buncombe Street

Project Area 2B



Modeled as part of the west side of Project 2A, only a portion of this network was deemed inadequate. The portion of the network from INLE 520 to INLE 485 is considered Project Area 2B.

Recommendation: The culvert crossing the road, GRAV 612 is recommended to be upgraded up through manhole STMH18.

PRELIMINARY COST OPINIONS FOR BUDGETING PURPOSES

In order to help the City of Hendersonville budget for the potential projects identified above, rudimentary cost opinions were developed for each project. The cost opinions were based on replacing the current infrastructure quantities only; costs associated with surveying, engineering design, environmental permitting, and land or easement acquisitions were not considered. Assumptions for proposed pipe size and location improvements were based on the conveyance capacity analysis and do not consider constraints such as existing utilities. Construction costs were assumed to be two times the materials cost. The unit pricing assumed concrete structures at 10-foot depth. All pipes were replaced with Class III Reinforced Concrete Pipe. Additional contingencies were added for budgeting purposes. Below is a summary of probable costs including these contingencies. Detailed costs assumptions are included with the project details section of this report.

Potential Project	Probable Costs
Project Area 1A	\$1,380,000
Project Area 1B	\$930,000
Project Area 1C	\$640,000
Project Area 2A	\$1,130,000
Project Area 2B	\$270,000

RECOMMENDATIONS

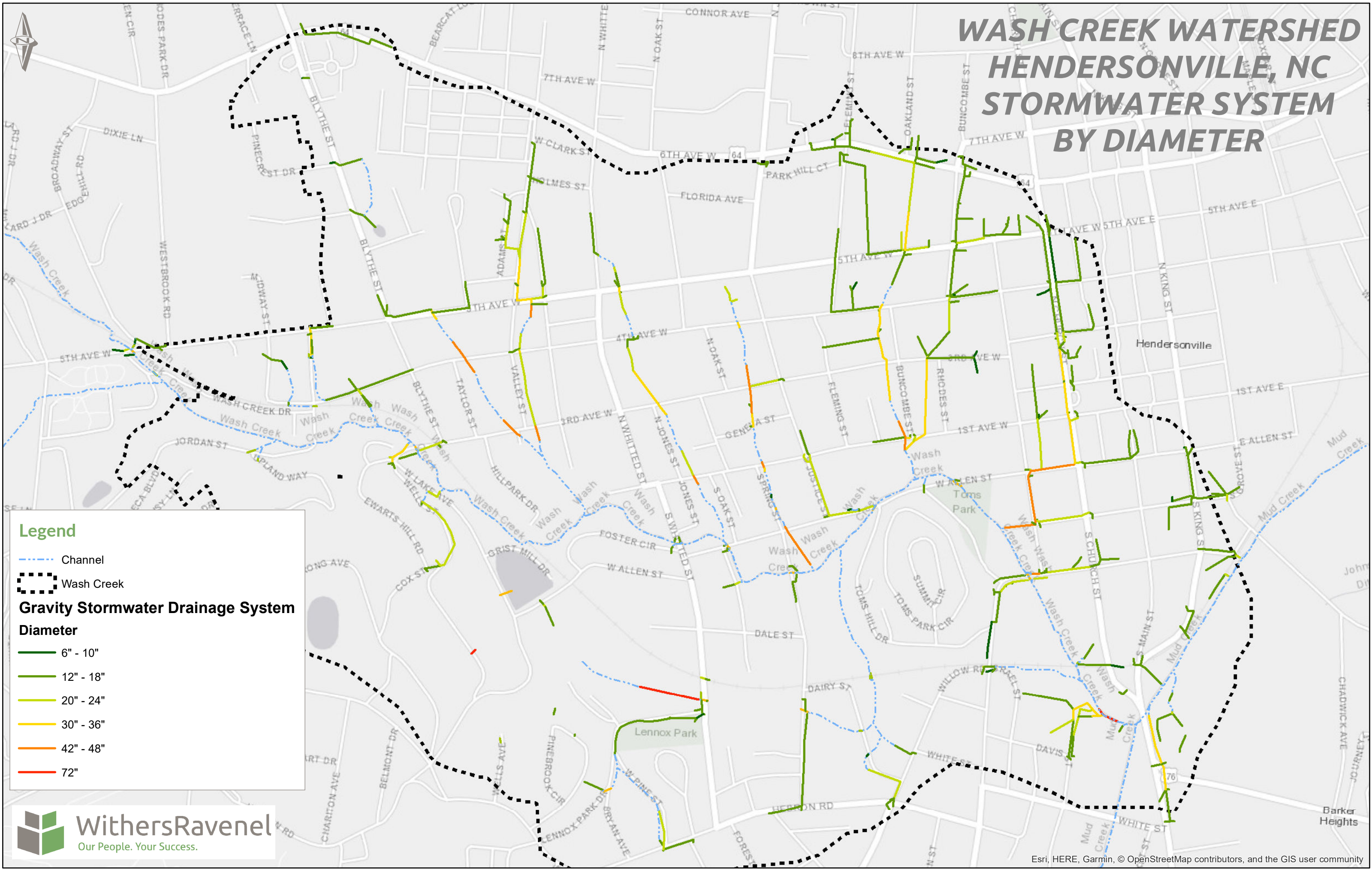
Five potential project areas were identified based on the prioritization analysis and areas of concern identified through discussions with the City. Of the five areas of concern identified by the City, two contained the highest priority areas. Three potential projects were identified in Area of Concern 1 and two potential project areas were identified in Area of Concern 2. Our preliminary hydraulic analysis of these systems identified them as being undersized for the design storm. Therefore, it is recommended that the improvements to these areas also account for improvements to the hydraulic capacity of the systems as well. Preliminary budget information has been provided to help assist in the prioritization of these projects. Due to the nature of the high priority factors relative to the estimated costs, it is recommended that Project Areas be explored in the following order 1C, 2A, 1B, 2B, 1A.

Regular maintenance is recommended for individual structures considered to be in need of repair. Fifteen structures were identified during the field reconnaissance as requiring immediate attention. Not all of these structures were captured by the recommend improvement projects.

The prioritization analysis identified areas outside of the five areas of concern that could potentially require attention in the near future. These areas were identified in this study but were not analyzed as they were outside of the areas of concern. It is recommended that these areas be monitored and studied further to determine any deficiencies in the network and to determine possible improvements.

INVENTORY ANALYSIS MAPS

WASH CREEK WATERSHED HENDERSONVILLE, NC STORMWATER SYSTEM BY DIAMETER



Legend

- - - Channel
- Wash Creek

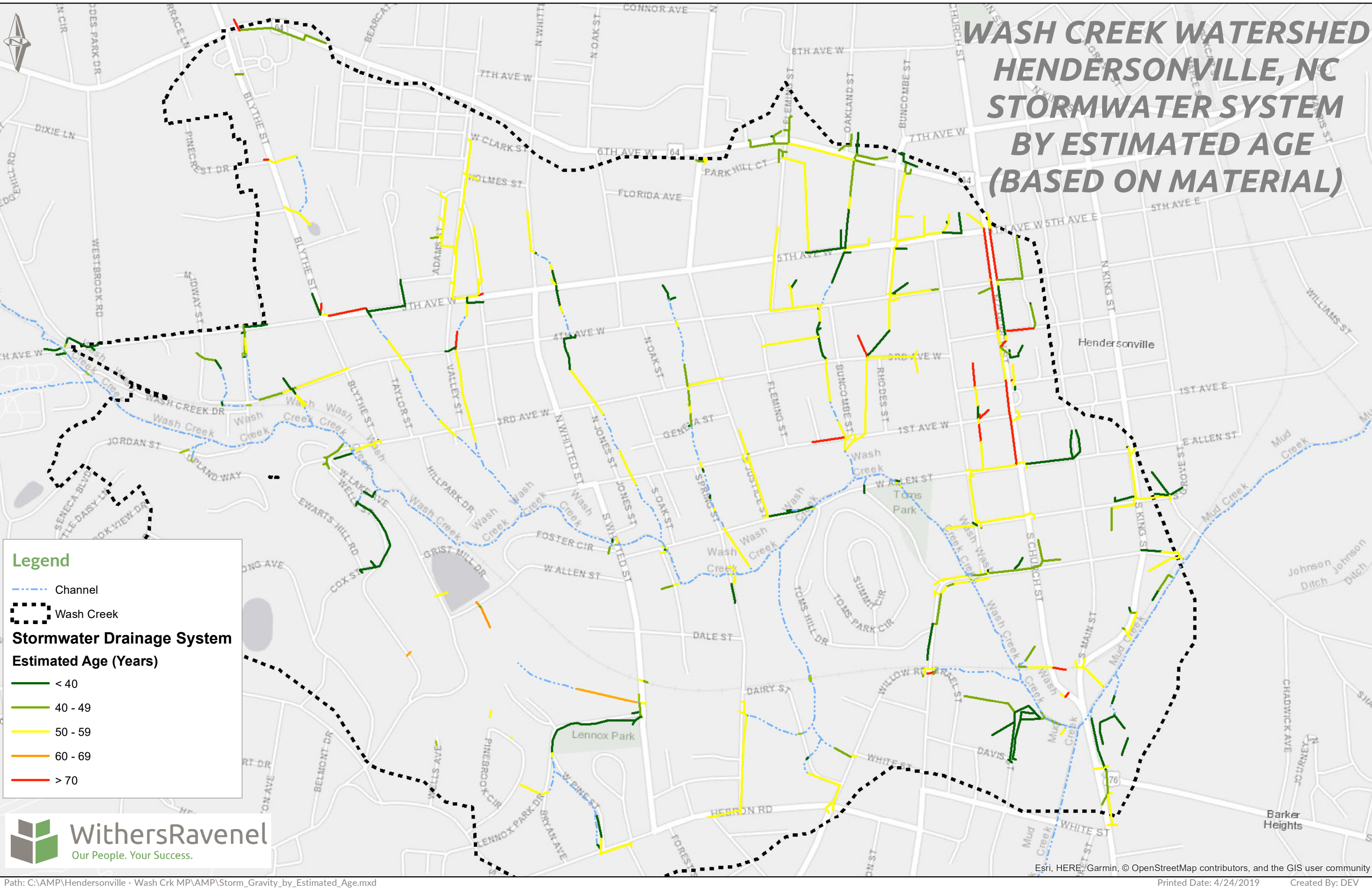
Gravity Stormwater Drainage System

Diameter

- 6" - 10"
- 12" - 18"
- 20" - 24"
- 30" - 36"
- 42" - 48"
- 72"



WASH CREEK WATERSHED HENDERSONVILLE, NC STORMWATER SYSTEM BY ESTIMATED AGE (BASED ON MATERIAL)



Legend

- Channel
- Wash Creek

Stormwater Drainage System

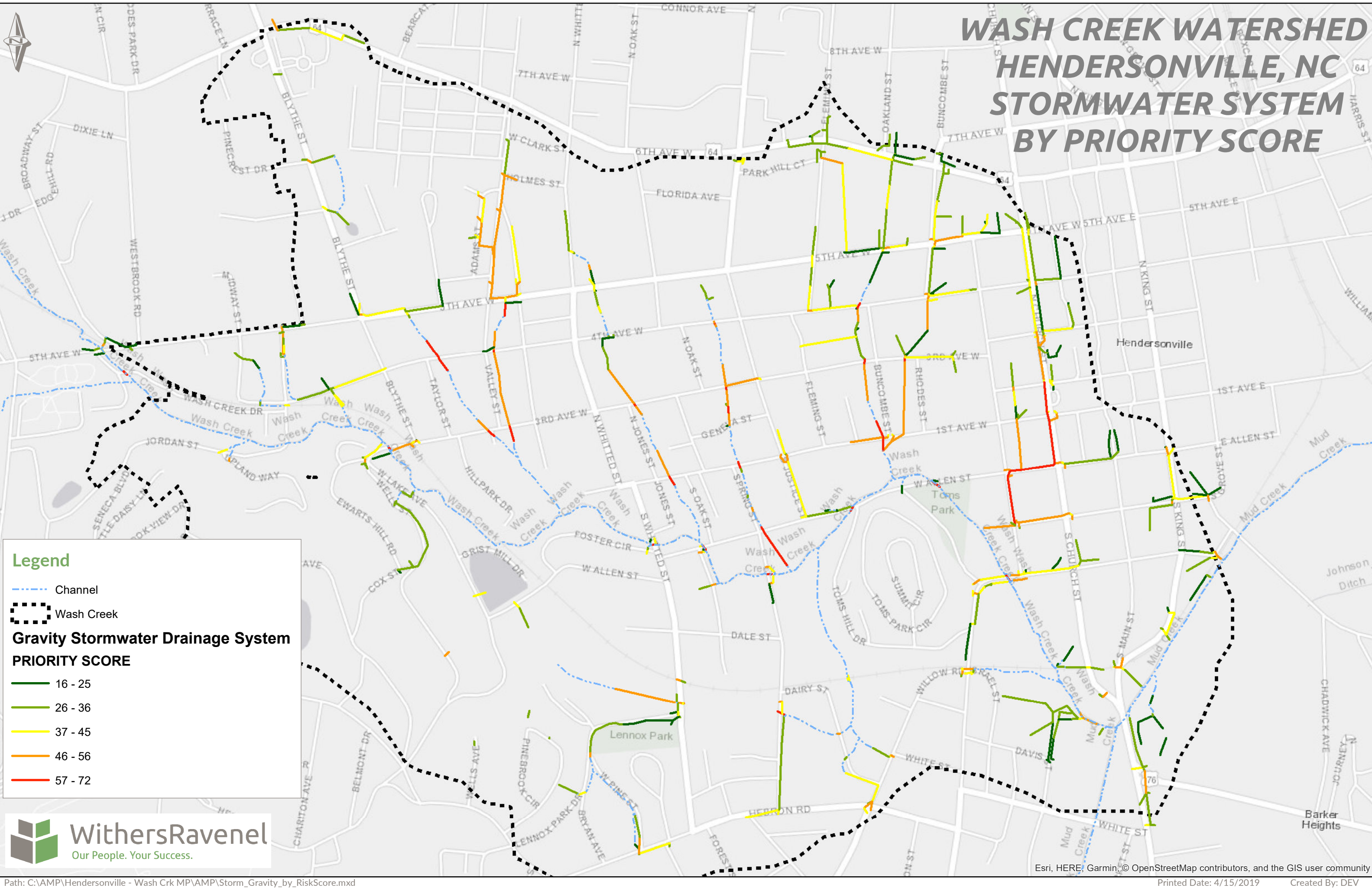
Estimated Age (Years)

- < 40
- 40 - 49
- 50 - 59
- 60 - 69
- > 70



PRIORITIZATION MAP

WASH CREEK WATERSHED HENDERSONVILLE, NC STORMWATER SYSTEM BY PRIORITY SCORE



Legend

- Channel
- Wash Creek

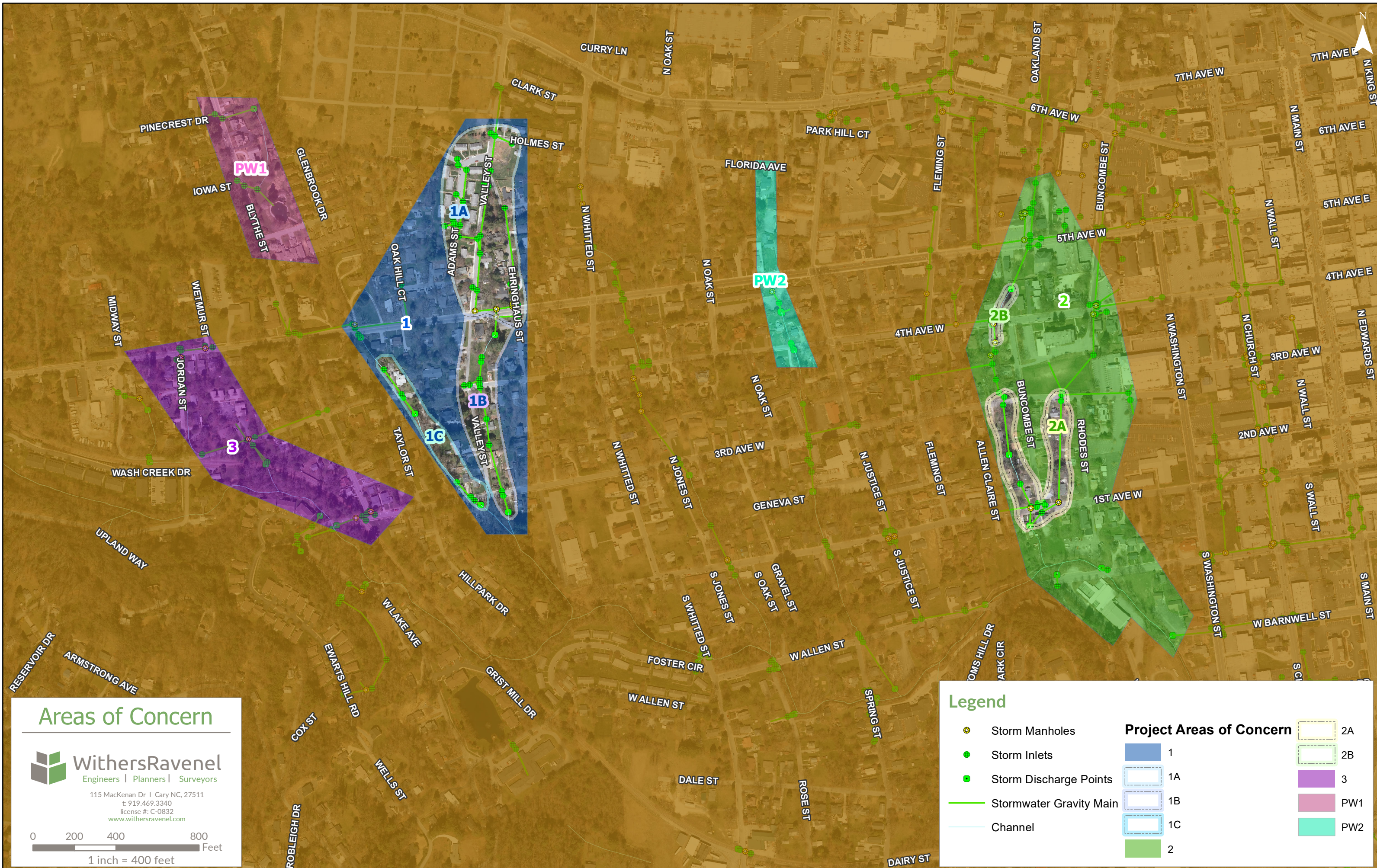
Gravity Stormwater Drainage System

PRIORITY SCORE

- 16 - 25
- 26 - 36
- 37 - 45
- 46 - 56
- 57 - 72



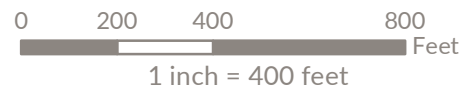
AREAS OF CONCERN



Areas of Concern



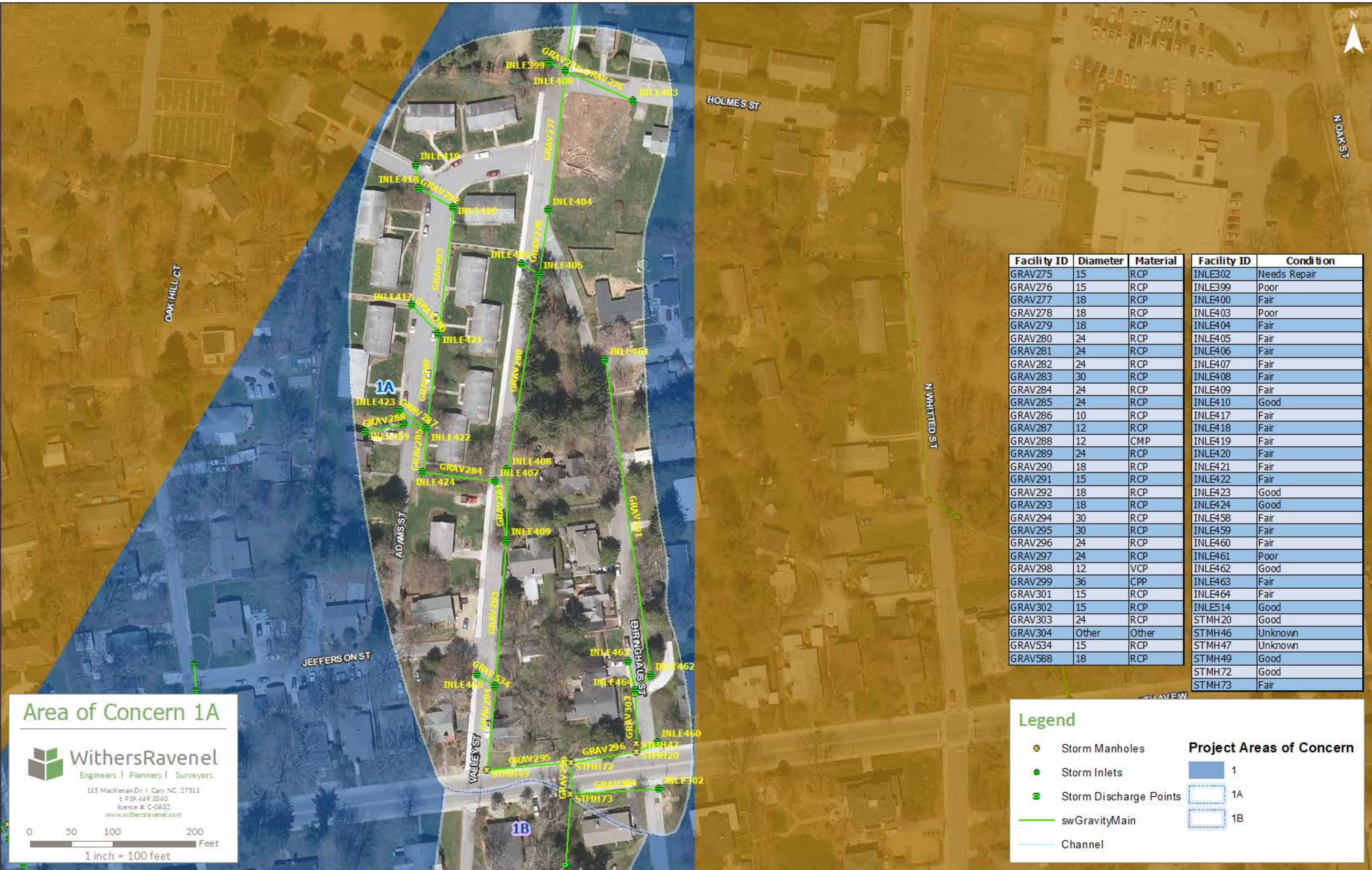
115 MacKenan Dr | Cary NC, 27511
t: 919.469.3340
license #: C-0832
www.withersravenel.com



Legend

- Storm Manholes
 - Storm Inlets
 - Storm Discharge Points
 - Stormwater Gravity Main
 - Channel
- | Project Areas of Concern | |
|--------------------------|-----|
| | 1 |
| | 1A |
| | 1B |
| | 1C |
| | 2 |
| | 2A |
| | 2B |
| | 3 |
| | PW1 |
| | PW2 |

PROJECT AREA ANALYSIS



Area of Concern 1A

WithersRavenel
 Engineers | Planners | Surveyors
 115 MacKenan Dr | Cary NC 27511
 t 919.469.3340
 license # C-0832
 www.withersravenel.com

0 50 100 200
 Feet
 1 inch = 100 feet

Facility ID	Diameter	Material	Facility ID	Condition
GRAV275	15	RCP	INLE302	Needs Repair
GRAV276	15	RCP	INLE399	Poor
GRAV277	18	RCP	INLE400	Fair
GRAV278	18	RCP	INLE403	Poor
GRAV279	18	RCP	INLE404	Fair
GRAV280	24	RCP	INLE405	Fair
GRAV281	24	RCP	INLE406	Fair
GRAV282	24	RCP	INLE407	Fair
GRAV283	30	RCP	INLE408	Fair
GRAV284	24	RCP	INLE409	Fair
GRAV285	24	RCP	INLE410	Good
GRAV286	10	RCP	INLE417	Fair
GRAV287	12	RCP	INLE418	Fair
GRAV288	12	CMP	INLE419	Fair
GRAV289	24	RCP	INLE420	Fair
GRAV290	18	RCP	INLE421	Fair
GRAV291	15	RCP	INLE422	Fair
GRAV292	18	RCP	INLE423	Good
GRAV293	18	RCP	INLE424	Good
GRAV294	30	RCP	INLE458	Fair
GRAV295	30	RCP	INLE459	Fair
GRAV296	24	RCP	INLE460	Fair
GRAV297	24	RCP	INLE461	Poor
GRAV298	12	VCP	INLE462	Good
GRAV299	36	CPP	INLE463	Fair
GRAV301	15	RCP	INLE464	Fair
GRAV302	15	RCP	INLE514	Good
GRAV303	24	RCP	STMH20	Good
GRAV304	Other	Other	STMH46	Unknown
GRAV534	15	RCP	STMH47	Unknown
GRAV588	18	RCP	STMH49	Good
			STMH72	Good
			STMH73	Fair

Legend

- Storm Manholes
- Storm Inlets
- Storm Discharge Points
- swGravityMain
- Channel

Project Areas of Concern

- 1
- 1A
- 1B

Time: 4/5/2019 12:10:00 AM

HGL

Conduit GRAV301
Flow = -3.277 cfs

Conduit GRAV588
Flow = 26.824 cfs

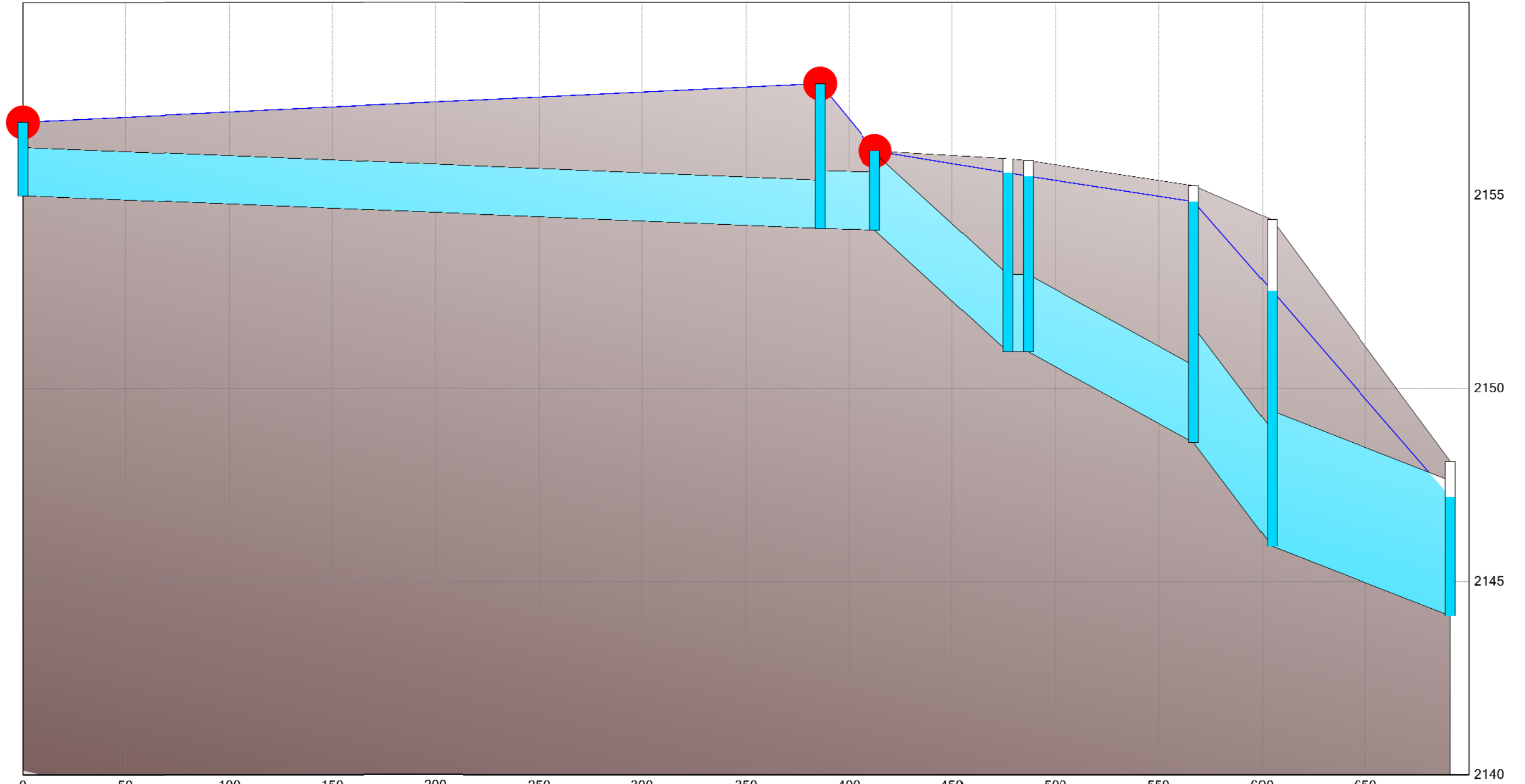
Conduit GRAV303
Flow = 21.286 cfs

Conduit GRAV297
Flow = 21.286 cfs

Conduit GRAV296
Flow = 20.477 cfs

Conduit GRAV299
Flow = 10.727 cfs

Conduit GRAV300
Flow = 17.474 cfs



Junction INLE461
CWSEL = 2156.9 ft

Junction INLE462
CWSEL = 2157.893 ft

Junction INLE464
CWSEL = 2156.15 ft

Junction STMH47
CWSEL = 2155.582 ft

Junction STMH20
CWSEL = 2155.494 ft

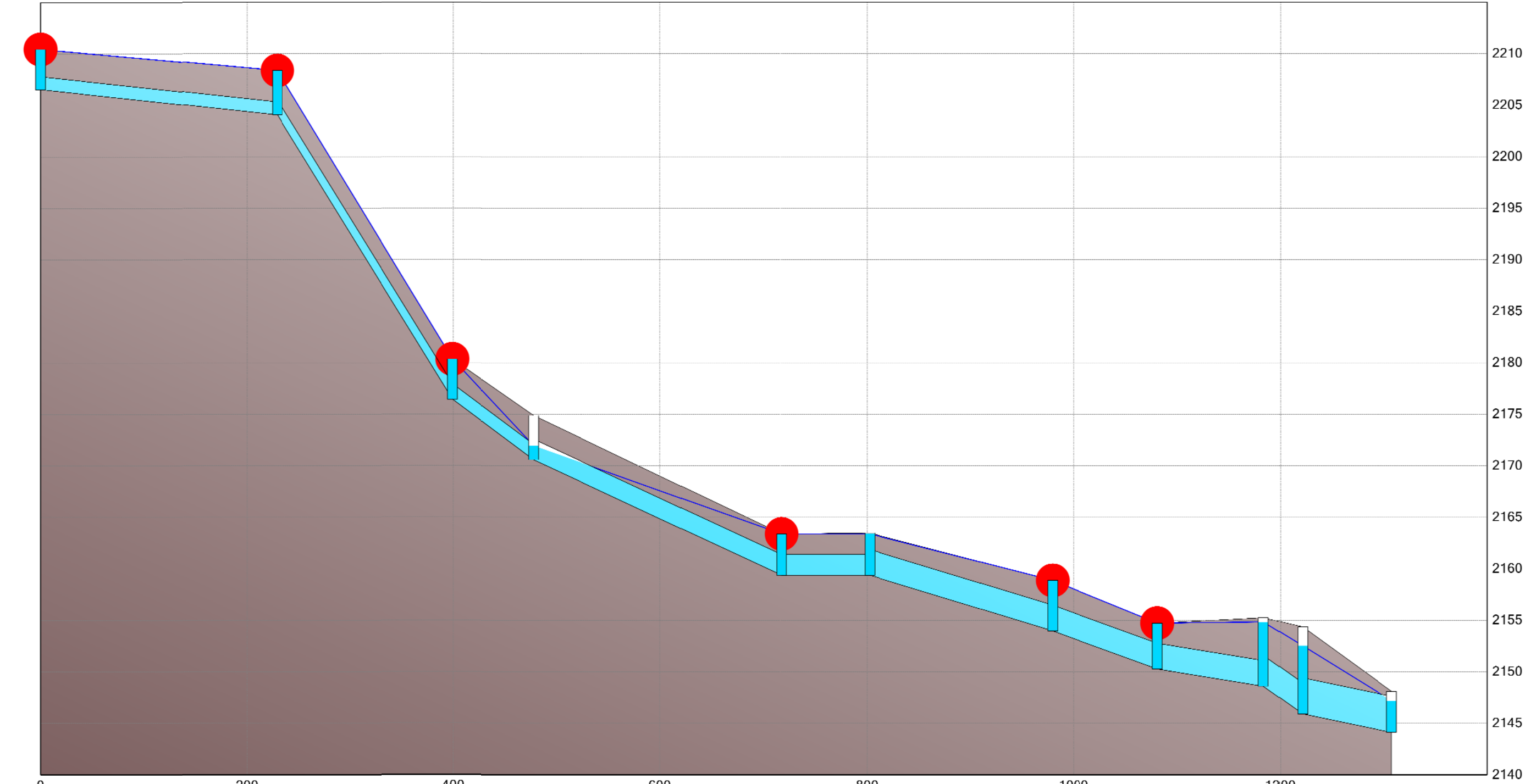
Junction STMH72
CWSEL = 2154.837 ft

Junction STMH73
CWSEL = 2152.526 ft

Junction STDP33
CWSEL = 2147.195 ft

HGL

Links: GRAV274 Q=6.163 cfs GRAV277 Q=42.685 cfs GRAV278 Q=34.159 cfs GRAV280 Q=38.453 cfs GRAV281 Q=-2.869 cfs GRAV283 Q=65.577 cfs GRAV294 Q=82.93 cfs GRAV295 Q=-13.475 cfs GRAV299 Q=10.727 cfs GRAV300 Q=17.474 cfs



Nodes: INLE401 H=2210.455 ft INLE400 H=2208.366 ft INLE404 H=2180.349 ft INLE405 H=2171.952 ft INLE408 H=2163.382 ft INLE409 H=2163.396 ft INLE410 H=2158.872 ft STMH49 H=2154.726 ft STMH72 H=2154.837 ft STMH73 H=2152.526 ft STDP33 H=2147.195 ft

Project Area 1A
Existing System

Structure	Quantity (pipes in lf)
15" RCP	730
18" RCP	548
24" RCP	917
30" RCP	450
36" RCP	381
Manholes	6
Inlets	27
Headwalls	1

Proposed System

Structure	Quantity (pipes in lf)	Unit Price	Total
24" RCP	1278	115	146929
30" RCP	917	125	114674
36" RCP	450	150	67500
Manholes	6	3600	21600
Inlets	27	3600	97200
Headwalls	1	5000	5000
Materials Subtotal			\$452,903.89
With Cost of Construction			\$910,000.00
With Engineering and Survey Costs			\$1,100,000.00
Additional Contingency			\$1,380,000.00

Assumptions

Does not include replacement of downstream culvert (GRAV299, STDP33)

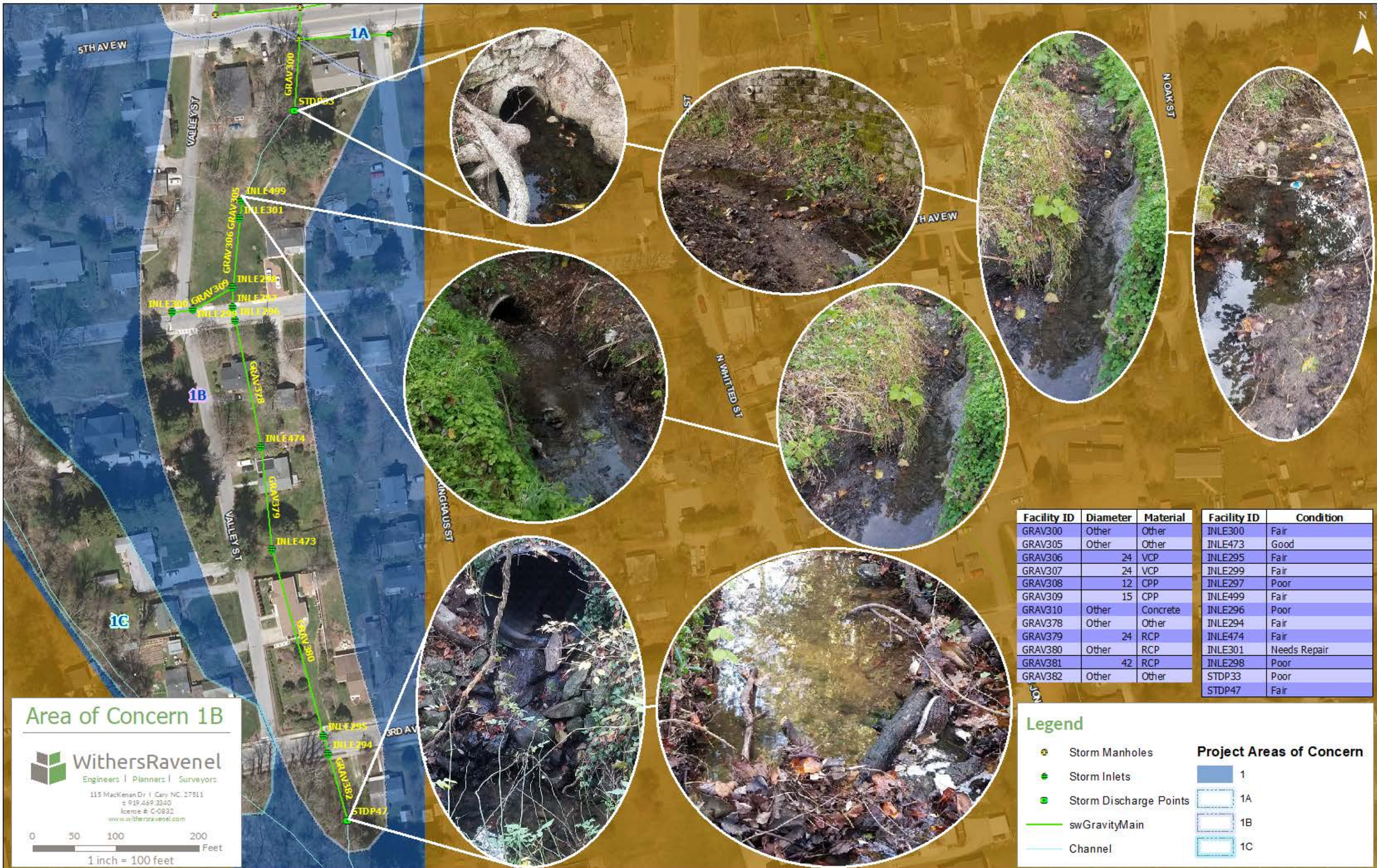
All structures assumed to have a 10' depth

All pipes replaced with Class III RCP (assumed depth 10') minimum pipe size is 15"

Construction costs assumed to be 2x materials

Engineering and Survey Costs assumed to be 20% of Construction Costs

25% additional contingency



Area of Concern 1B

WithersRavenel
 Engineers | Planners | Surveyors
 115 MacKenan Dr | Cary NC 27511
 ☎ 919.469.3340
 license #: C-0832
 www.withersravenel.com

0 50 100 200 Feet
 1 inch = 100 feet

Facility ID	Diameter	Material	Facility ID	Condition
GRAV300	Other	Other	INLE300	Fair
GRAV305	Other	Other	INLE473	Good
GRAV306	24	VCP	INLE295	Fair
GRAV307	24	VCP	INLE299	Fair
GRAV308	12	CPP	INLE297	Poor
GRAV309	15	CPP	INLE499	Fair
GRAV310	Other	Concrete	INLE296	Poor
GRAV378	Other	Other	INLE294	Fair
GRAV379	24	RCP	INLE474	Fair
GRAV380	Other	RCP	INLE301	Needs Repair
GRAV381	42	RCP	INLE298	Poor
GRAV382	Other	Other	STDP33	Poor
			STDP47	Fair

Legend

- Storm Manholes
- Storm Inlets
- Storm Discharge Points
- swGravityMain
- Channel

Project Areas of Concern

- 1
- 1A
- 1B
- 1C

— HGL

Links: C1
Q=17.456 cfs

GRAV305
Q=2.43 cfs

GRAV306
Q=2.43 cfs

GRAV307
Q=-3.395 cfs

GRAV310
Q=20.402 cfs

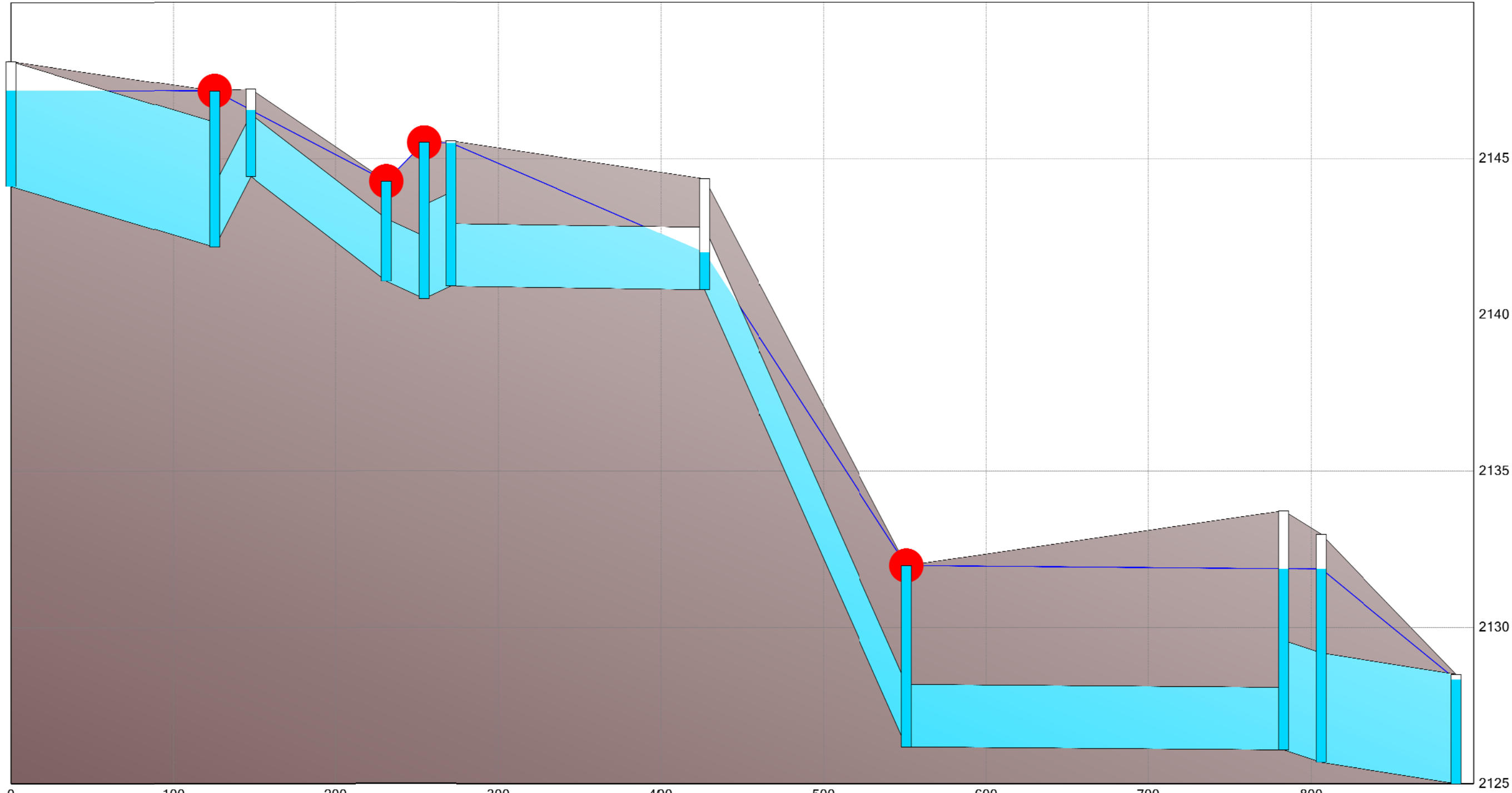
GRAV378
Q=20.402 cfs

GRAV379
Q=50.37 cfs

GRAV380
Q=4.796 cfs

GRAV381
Q=4.796 cfs

GRAV382
Q=136.589 cfs



Nodes: STDP33
H=2147.195 ft

INLE499
H=2147.162 ft

INLE301
H=2146.549 ft

INLE298
H=2144.268 ft

INLE297
H=2145.508 ft

INLE296
H=2145.492 ft

INLE474
H=2142 ft

INLE473
H=2131.98 ft

INLE295
H=2131.876 ft

INLE294
H=2131.875 ft

STDP47
H=2128.332 ft

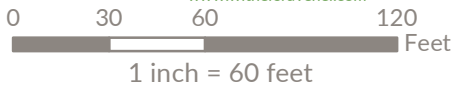


Proposed Alignment Area of Concern 1B



WithersRavenel
Engineers | Planners | Surveyors

115 MacKenan Dr | Cary NC, 27511
t: 919.469.3340
license #: C-0832
www.withersravenel.com



Legend

- Proposed Alignment
- ⊗ Storm Manholes
- Storm Inlets
- ⊗ Storm Discharge Points
- swGravityMain
- Channel

Project Area 1B
Existing System

Structure	Quantity (pipes in lf)
24" RCP	231
42" RCP	23
Small Box Culvert	596
Inlets	11
Headwalls	2

Proposed System

Structure	Quantity (pipes in lf)	Unit Price	Total
42" RCP	331	180	59670
Small Box Culvert	619	300	185636
Inlets	13	3600	46800
Headwalls	2	5000	10000
Materials Subtotal			\$302,105.75
With Cost of Construction			\$610,000.00
With Engineering and Survey Costs			\$740,000.00
Additional Contingency			\$930,000.00

Assumptions

Includes additional 100 lf of pipe and 2 additional structures

All structures assumed to have a 10' depth

All pipes replaced with Class III RCP (assumed depth 10') minimum pipe size is 15"

Construction costs assumed to be 2x materials

Engineering and Survey Costs assumed to be 20% of Construction Costs

25% additional contingency



Facility ID	Diameter	Material	Facility ID	Condition
GRAV383	48"	RCP	INLE381	Poor
GRAV384	Other	Other	INLE382	Fair
GRAV385	48"	RCP	INLE383	Good
GRAV386	Other	Other	INLE384	Good
GRAV555	Other	Concrete	INLE436	Good
GRAV604	Other	Concrete	INLE507	Good
GRAV605	Other	Other	INLE515	Poor
			STDP62	Good
			STDP63	Poor

Area of Concern 1C

WithersRavenel
Engineers | Planners | Surveyors

115 MacKenan Dr | Cary, NC 27511
t 919.469.3340
license #: C-0832
www.withersravenel.com

0 50 100 200 Feet
1 inch = 100 feet

Legend

- Storm Manholes
- Storm Inlets
- Storm Discharge Points
- Stormwater Gravity Main
- Channel

Project Areas of Concern

- 1
- 1B
- 1C
- 3

Time: 4/5/2019 12:15:10 AM

HGL

Conduit GRAV555
Flow = 212.444 cfs

Conduit GRAV604
Flow = 111.415 cfs

Conduit GRAV605
Flow = 10.532 cfs

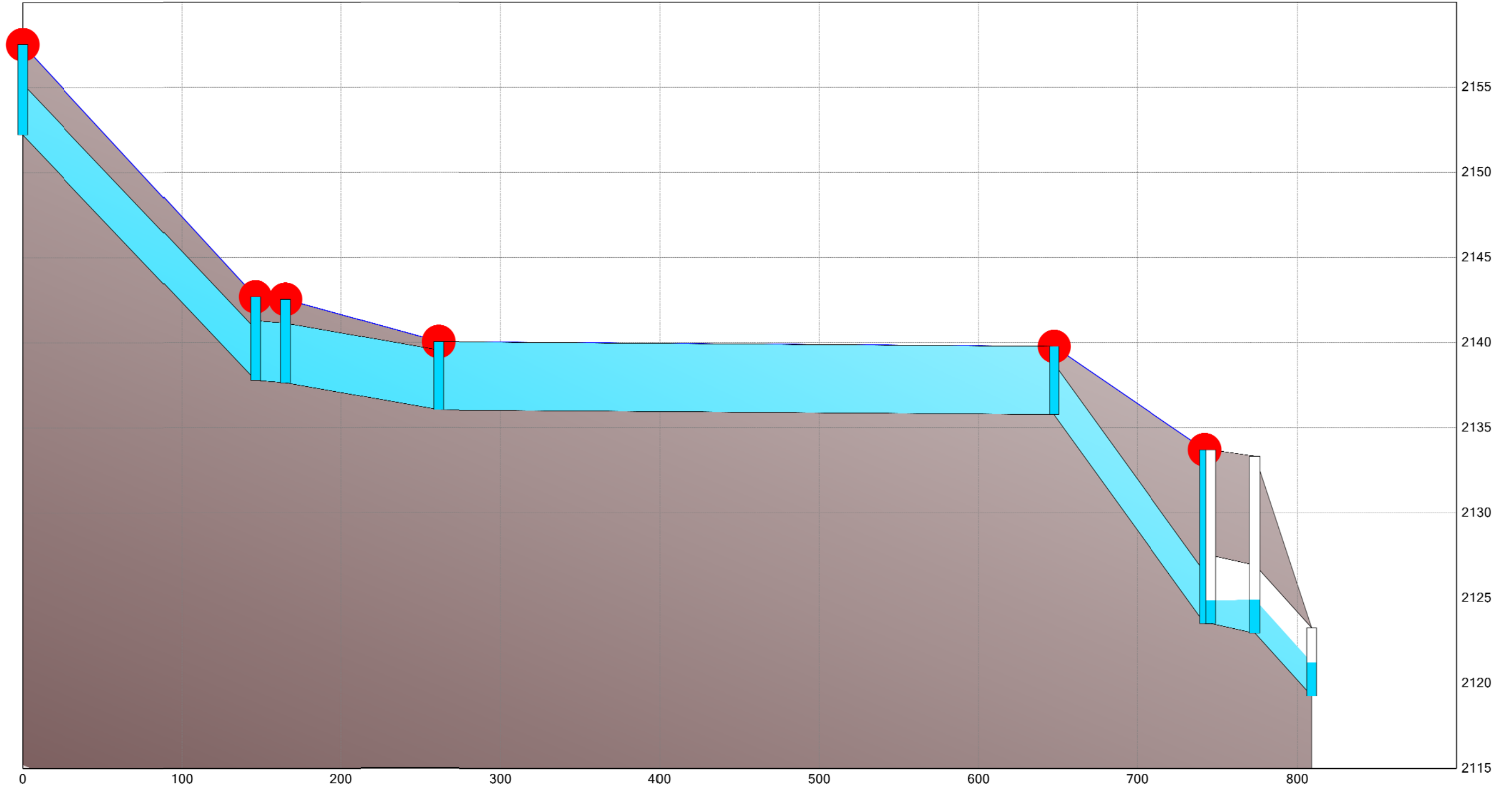
Conduit C2
Flow = 57.368 cfs

Conduit GRAV386
Flow = 10.982 cfs

Conduit GRAV384
Flow = -10.397 cfs

Conduit GRAV385
Flow = -10.397 cfs

Conduit GRAV383
Flow = 225.13 cfs



Junction INLE515
CWSEL = 2157.533 ft

Junction INLE382
CWSEL = 2142.694 ft

Junction INLE381
CWSEL = 2142.55 ft

Junction STDP63
CWSEL = 2140.055 ft

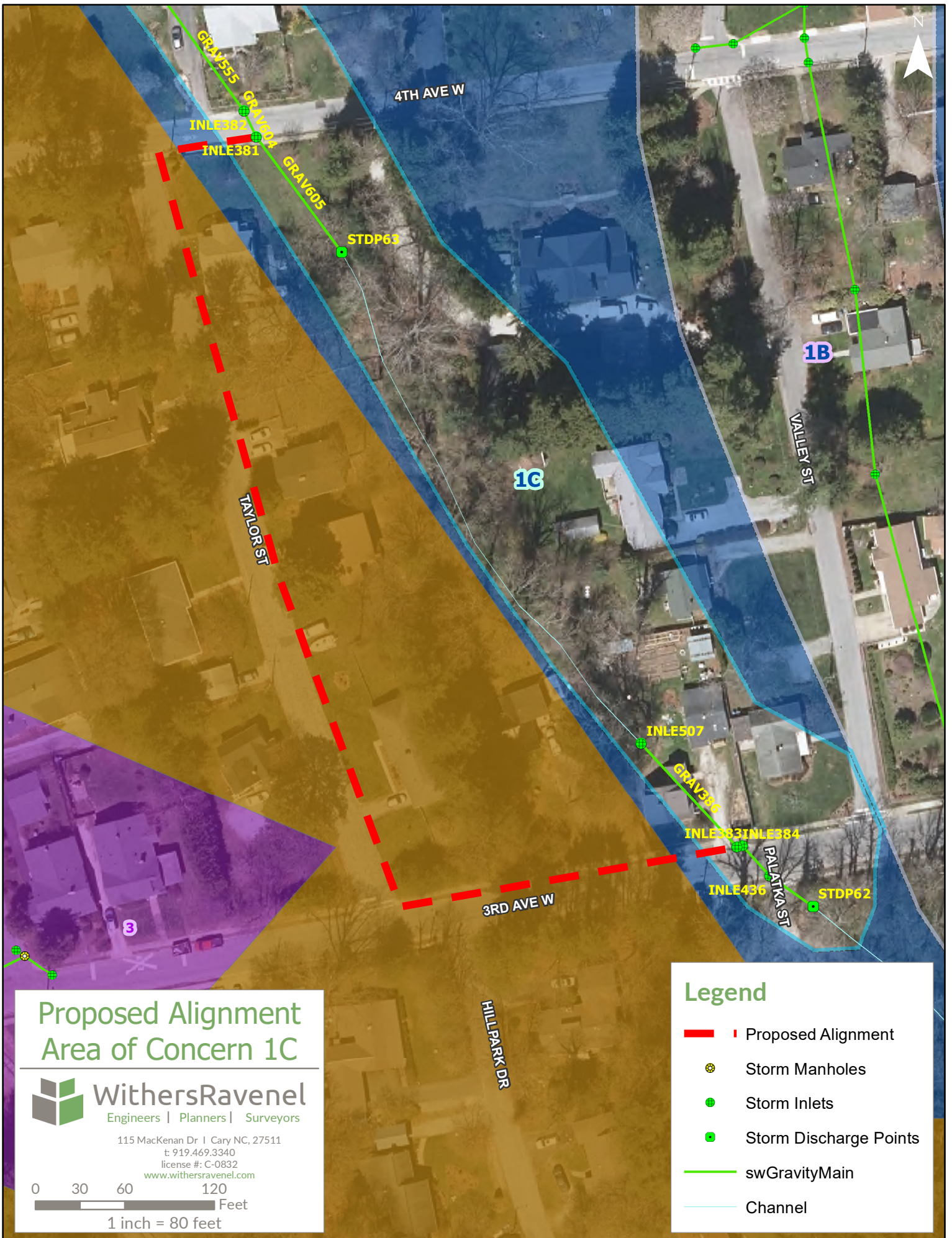
Junction INLE507
CWSEL = 2139.785 ft

Junction INLE384
CWSEL = 2133.717 ft

Junction INLE383
CWSEL = 2124.857 ft

Junction INLE436
CWSEL = 2124.916 ft

Outfall STDP62
CWSEL = 2121.227 ft

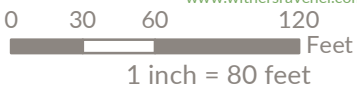


Proposed Alignment Area of Concern 1C



WithersRavenel
Engineers | Planners | Surveyors

115 MacKenan Dr | Cary NC, 27511
t: 919.469.3340
license #: C-0832
www.withersravenel.com



Legend

- Proposed Alignment
- ⊗ Storm Manholes
- Storm Inlets
- Storm Discharge Points
- swGravityMain
- Channel

Project Area 1C
Existing System

Structure	Quantity (pipes in lf)
48" Pipe	63
Small Box Culvert	360
Inlets	7
Headwalls	2

Proposed System

Structure	Quantity (pipes in lf)	Unit Price	Total
48" RCP	800	200	160000
Inlets	11	3600	39600
Headwalls	2	5000	10000
Materials Subtotal			\$209,600.00
With Cost of Construction			\$420,000.00
With Engineering and Survey Costs			\$510,000.00
Additional Contingency			\$640,000.00

Assumptions

Includes additional 800 lf of pipe of new pipe and 2 additional structures

Does not include stream improvements

All structures assumed to have a 10' depth

All pipes replaced with Class III RCP (assumed depth 10') minimum pipe size is 15"

Construction costs assumed to be 2x materials

Engineering and Survey Costs assumed to be 20% of Construction Costs

25% additional contingency

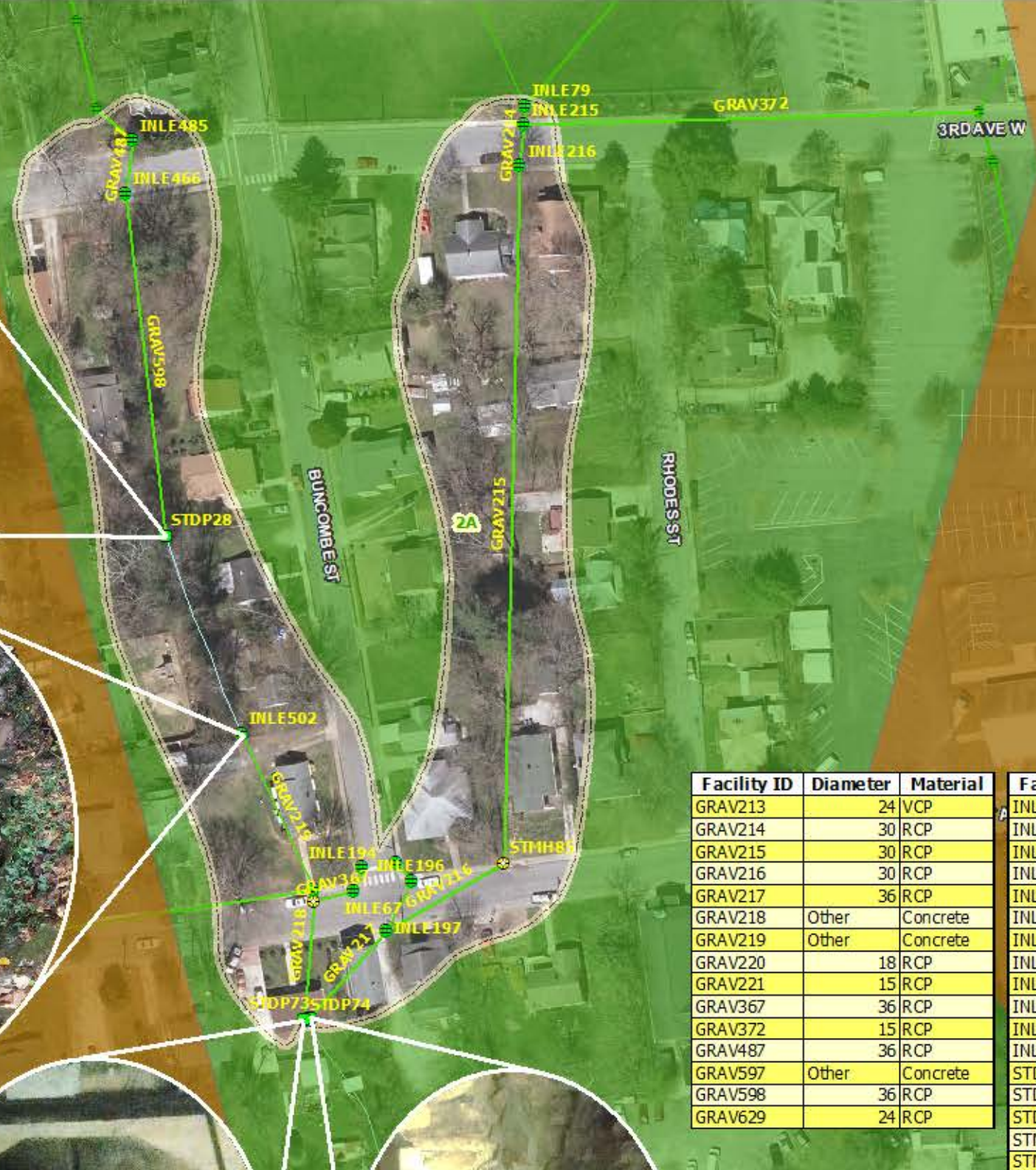


N JUSTICE ST

GENEVA ST

FLEMING ST

S JUSTICE ST



Facility ID	Diameter	Material	Facility ID	Condition
GRAV213	24	VCP	INLE67	Fair
GRAV214	30	RCP	INLE79	Poor
GRAV215	30	RCP	INLE193	Fair
GRAV216	30	RCP	INLE194	Poor
GRAV217	36	RCP	INLE195	Fair
GRAV218	Other	Concrete	INLE196	Fair
GRAV219	Other	Concrete	INLE197	Fair
GRAV220	18	RCP	INLE215	Good
GRAV221	15	RCP	INLE216	Poor
GRAV367	36	RCP	INLE466	Fair
GRAV372	15	RCP	INLE485	Fair
GRAV487	36	RCP	INLE502	Poor
GRAV597	Other	Concrete	STDP28	Poor
GRAV598	36	RCP	STDP73	Fair
GRAV629	24	RCP	STDP74	Fair
			STMH85	Good
			STMH14	Unknown

Area of Concern 2A

WithersRavenel
Engineers | Planners | Surveyors

115 MacKenan Dr | Cary NC, 27511
t 919.469.3340
license #: C-0832
www.withersravenel.com

0 50 100 200 Feet
1 inch = 100 feet

Legend

- Storm Manholes
- Storm Inlets
- Storm Discharge Points
- Stormwater Gravity Main
- Channel

Project Areas of Concern

- 2
- 2A

HGL

Conduit GRAV207
Flow = 34.231 cfs

Conduit GRAV211
Flow = 12.395 cfs

Conduit GRAV212
Flow = 14.169 cfs

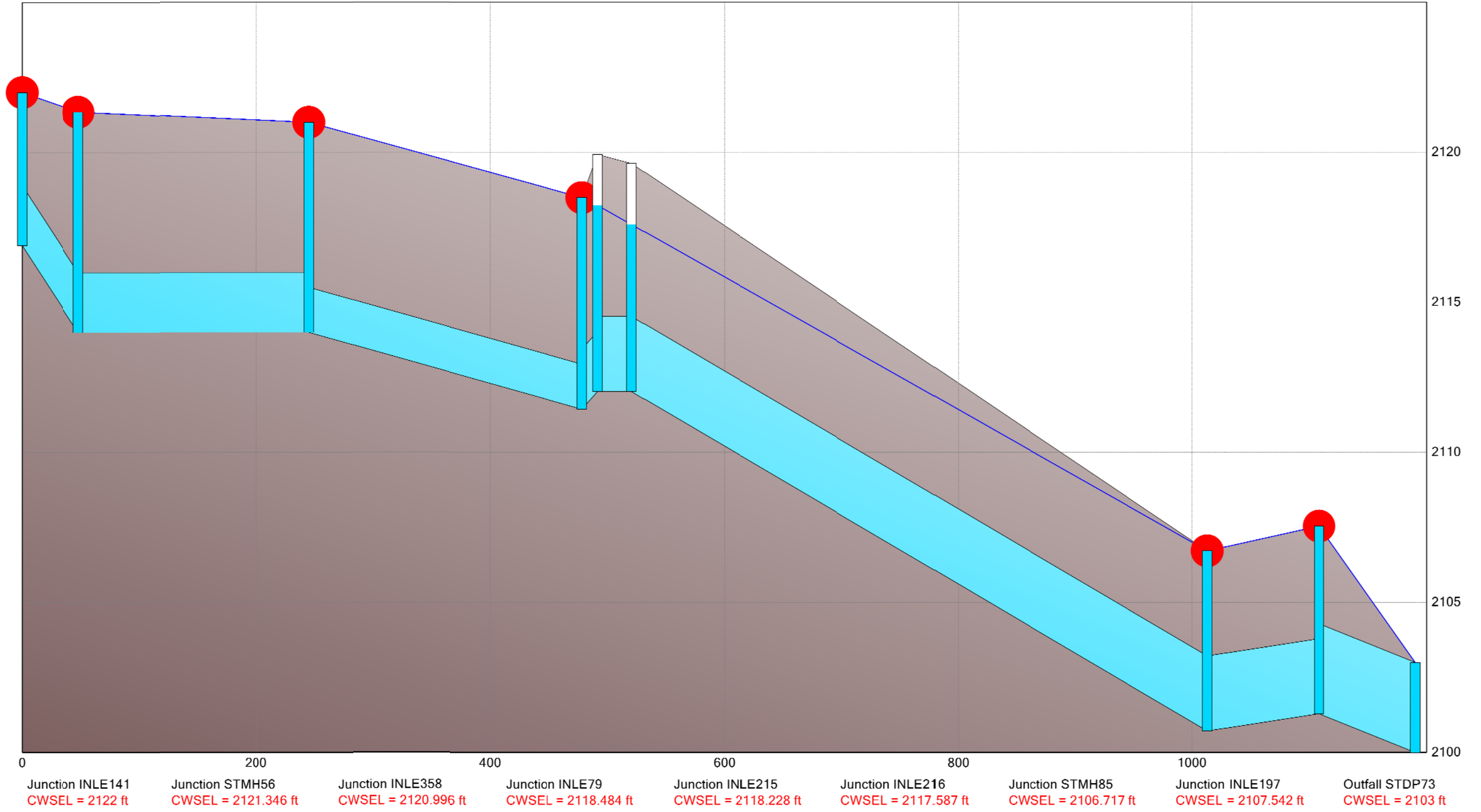
Conduit GRAV213
Flow = 40.557 cfs

Conduit GRAV214
Flow = 79.265 cfs

Conduit GRAV215
Flow = 79.265 cfs

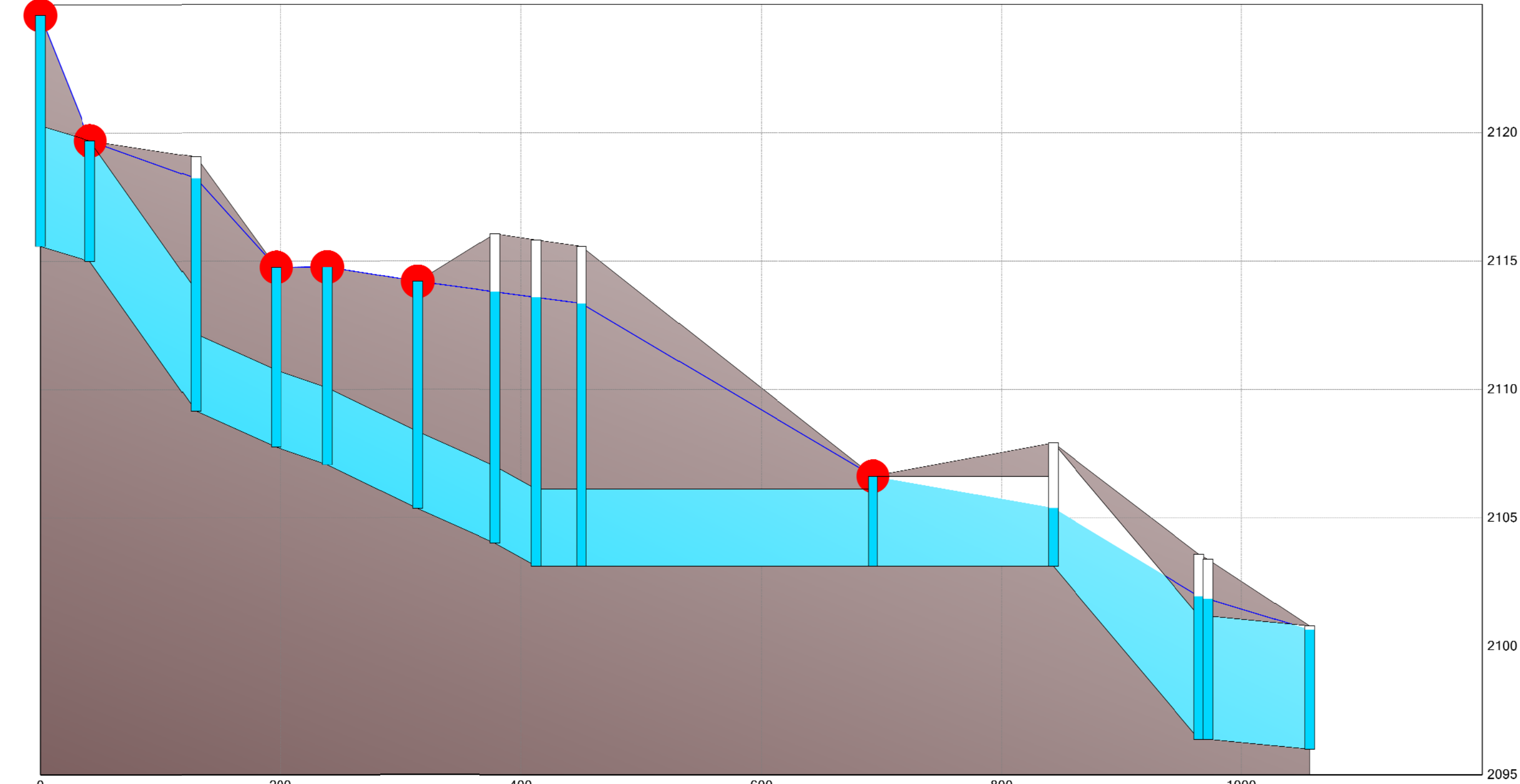
Conduit GRAV216
Flow = -49.619 cfs

Conduit GRAV217
Flow = 203.907 cfs

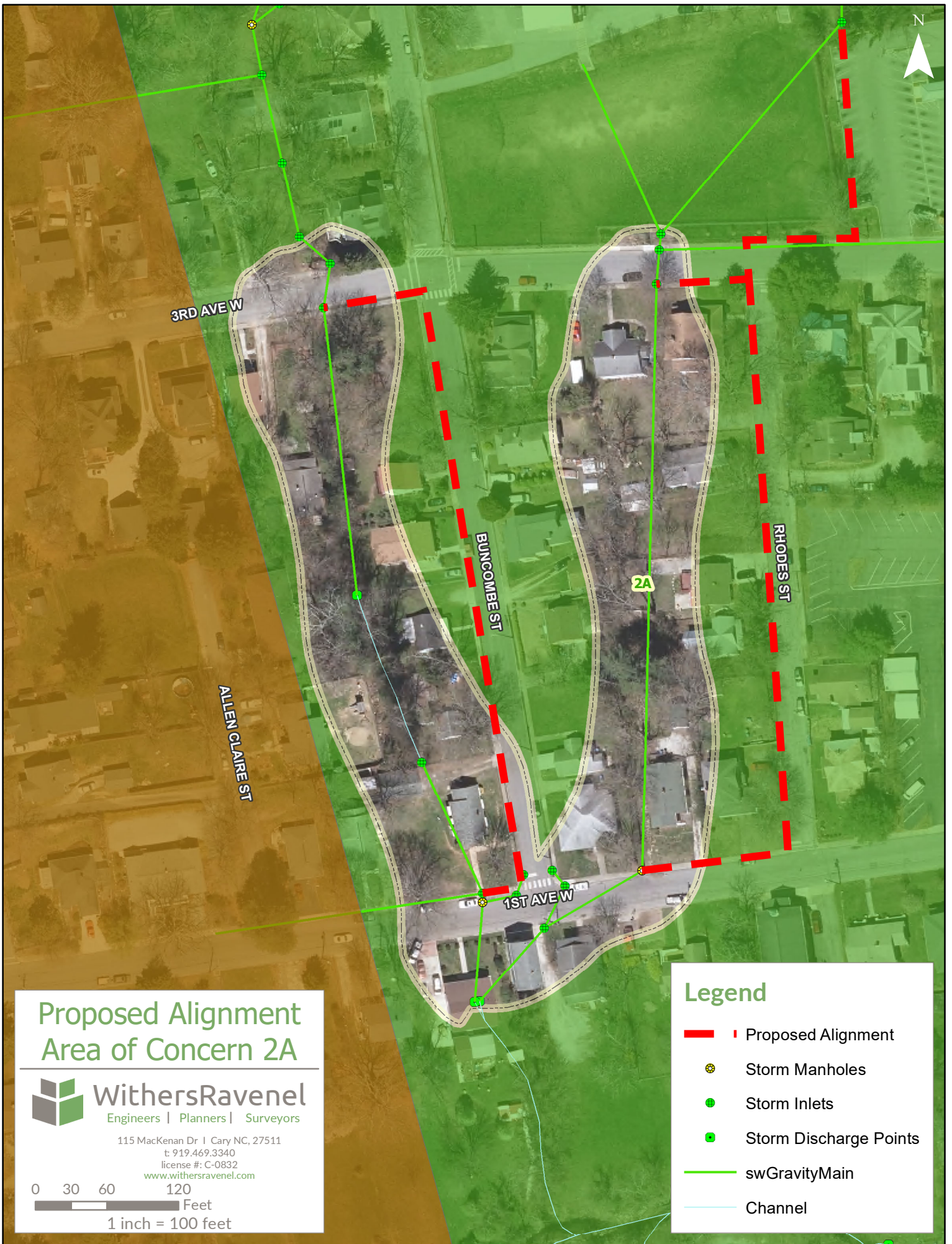


HGL

Links: GRAV612 Q=519.903 cfs GRAV610 Q=197.049 cfs GRAV374 Q=197.049 cfs GRAV375 Q=-15.707 cfs GRAV376 Q=75.249 cfs GRAV377 Q=68.676 cfs GRAV486 Q=68.676 cfs GRAV487 Q=68.676 cfs GRAV598 Q=144.343 cfs C3 Q=79.817 cfs GRAV219 Q=203.46 cfs GRAV597 Q=243.462 cfs GRAV218 Q=246.88 cfs



Nodes: INLE520 H=2124.589 ft STMH108 H=2119.7 ft STMH18 H=2118.225 ft STMH17 H=2114.759 ft INLE470 H=2114.773 ft INLE471 H=2114.205 ft INLE472 H=2113.805 ft INLE485 H=2113.59 ft INLE466 H=2113.352 ft STDP28 H=2106.628 ft INLE502 H=2105.389 ft INLE193 H=2101.948 ft STMH14 H=2101.845 ft STDP74 H=2100.656 ft



Proposed Alignment Area of Concern 2A



WithersRavenel
Engineers | Planners | Surveyors

115 MacKenan Dr | Cary NC, 27511
t: 919.469.3340
license #: C-0832
www.withersravenel.com

0 30 60 120
Feet
1 inch = 100 feet

Legend

- Proposed Alignment
- ⊗ Storm Manholes
- Storm Inlets
- ⊗ Storm Discharge Points
- swGravityMain
- Channel

Project Area 2a

Existing System

Structure	Quantity (pipes in lf)
15" Pipe	339
18" Pipe	17
24" Pipe	53
30" Pipe	616
36" Pipe	392
Small Box Culvert	213
Inlets	8

Proposed System

Structure	Quantity (pipes in lf)	Unit Price	Total
24" RCP	549	115	63144
30" RCP	53	125	6609
36" RCP	616	150	92428
42" RCP	363	180	65282
Small Box Culvert	483	200	96615
Inlets	13	3600	46800
Materials Subtotal			\$370,876.15
With Cost of Construction			\$750,000.00
With Engineering and Survey Costs			\$900,000.00
Additional Contingency			\$1,130,000.00

Assumptions

Includes realignment and upgrade of GRAV212 from 15" HDPE to 24" RCP (just outside of project area, upstream of INLE215)

Includes additional 195 lf of pipe and 4 additional structures on east and 270 lf of pipe and 1 additional structure on west

All structures assumed to have a 10' depth

All pipes replaced with Class III RCP (assumed depth 10') minimum pipe size is 15"

Construction costs assumed to be 2x materials

Engineering and Survey Costs assumed to be 20% of Construction Costs

25% additional contingency



Facility ID	Diameter	Material	Facility ID	Condition
GRAV610	Other	Concrete	INLE41	Poor
GRAV611	12	CPP	INLE520	Fair
GRAV612	Other	Concrete	STDP7	Poor
			STMH18	Excellent
			STMH108	Unknown

Area of Concern 2B

Engineers | Planners | Surveyors

115 MacKenan Dr | Cary NC 27511
 ☎ 919.469.3340
 license #: C-0832
 www.withersravenel.com

0 20 40 80
 Feet
 1 inch = 40 feet

Legend

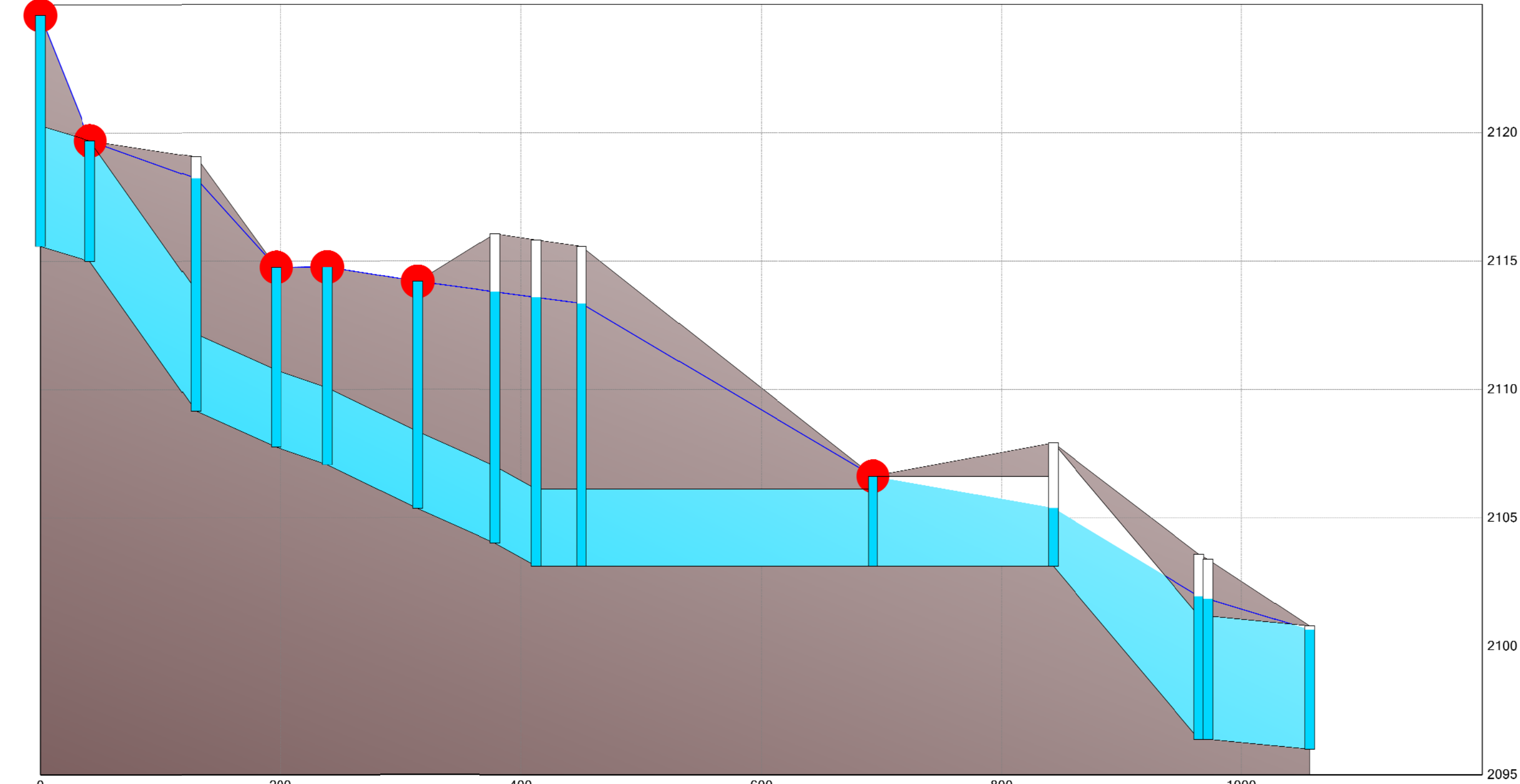
- Storm Manholes
- Storm Inlets
- Storm Discharge Points
- Stormwater Gravity Main
- Channel

Project Areas of Concern

- 2
- 2B

HGL

Links: GRAV612 Q=519.903 cfs GRAV610 Q=197.049 cfs GRAV374 Q=197.049 cfs GRAV375 Q=-15.707 cfs GRAV376 Q=75.249 cfs GRAV377 Q=68.676 cfs GRAV486 Q=68.676 cfs GRAV487 Q=68.676 cfs GRAV598 Q=144.343 cfs C3 Q=79.817 cfs GRAV219 Q=203.46 cfs GRAV597 Q=243.462 cfs GRAV218 Q=246.88 cfs



Nodes: INLE520 H=2124.589 ft STMH108 H=2119.7 ft STMH18 H=2118.225 ft STMH17 H=2114.759 ft INLE470 H=2114.773 ft INLE471 H=2114.205 ft INLE472 H=2113.805 ft INLE485 H=2113.59 ft INLE466 H=2113.352 ft STDP28 H=2106.628 ft INLE502 H=2105.389 ft INLE193 H=2101.948 ft STMH14 H=2101.845 ft STDP74 H=2100.656 ft

Project Area 2b
Existing System

Structure	Quantity (pipes in lf)
Large Box Culvert	129
Inlets	3
Headwalls	1

Proposed System

Structure	Quantity (pipes in lf)	Unit Price	Total
Large Box Culvert	129	500	64744
Inlets	3	3600	10800
Headwalls	1	5000	5000
Materials Subtotal			\$80,544.07
With Cost of Construction			\$170,000.00
With Engineering and Survey Costs			\$210,000.00
Additional Contingency			\$270,000.00

Assumptions

All structures assumed to have a 10' depth

All pipes replaced with Class III RCP (assumed depth 10') minimum pipe size is 15"

Construction costs assumed to be 2x materials

Engineering and Survey Costs assumed to be 20% of Construction Costs

25% additional contingency